



To: Prospective Bidders

From: Will Mastin  
City of Mobile Architectural Engineering Department

Re: Mims Park – Athletic Restrooms and Concession  
Project #PR-048-24B

Date: October 17, 2024

This Addendum forms a part of, and modifies, the Request for Bids, for the above referenced project, dated October 16, 2024. Acknowledge the receipt of this Addendum No. 1 in the space provided on the Bid Form. Failure to do so may subject Bidder to disqualification.

**General:**

**Clarifications:**

Item 1. The drawing set posted on the City of Mobile’s website on 10/16/2024 were missing the Structural Drawings - Sheet numbers S0.1 through S3.5 – a total of 14 pages of structural drawings. The missing drawing sheets are attached to this addendum.

Item 2. The Cover Sheet with the Sheet Index for this drawing set had some of the Electrical Plan sheet numbers labeled incorrectly. Please see the attached updated Cover Sheet with the correct Sheet Index numbers for Electrical Plans.

Item 3. The recommended pre-bid meeting for Bidders will be held at 10:00 am on October 23, 2024 at Mims Park at the site of the new Athletic Restrooms and Concessions – near field B at the Mims Park site – 5400 Grishilde Drive, Mobile, AL.

**Request for Bids:**

**Drawings:**

Item 1. Add the attached Structural Drawing sheets numbered S0.1, S0.2, S1.1, S1.2, S1.3, S1.4, S1.5, S1.6, S1.7, S3.1, S3.2, S3.3, S3.4, and S3.5 to the drawing set that was posted on City of Mobile's website on October 16, 2024.

Item 2. The Cover Sheet with the Sheet Index for this drawing set had some of the Electrical Plan sheet numbers labeled incorrectly. Replace the existing posted Cover Sheet with Sheet Index with the attached updated Cover Sheet with the correct Sheet Index numbers for Electrical Plans.

**RFI's:**

Question:

**END OF ADDENDUM NO. 1**



# MIMS PARK CONCESSION STAND & RESTROOMS

MOBILE, AL - PROJECT # PR-048-24B



## GENERAL NOTES:

1. PRIOR TO BIDDING, THE CONTRACTOR SHALL VISIT SITE AND THOROUGHLY FAMILIARIZE HIMSELF WITH ALL EXISTING CONDITIONS AND WITH THE CONTRACT DOCUMENTS ANY QUESTIONS OR DISCREPANCIES REGARDING THE NATURE OR INTENT OR THE WORK SHALL BE DIRECTED TO THE LANDSCAPE ARCHITECT PRIOR TO BIDDING.
2. ALL DEMOLITION AND REMOVAL WORK SHALL BE EXECUTED IN CONFORMANCE WITH ALL CODES AND ORDINANCES AS SET FORTH BY ALL GOVERNING AUTHORITIES.
3. CONTRACTOR IS RESPONSIBLE FOR ALL SAFETY ON THE PROJECT, AND SHALL TAKE ALL NECESSARY PRECAUTIONS TO MAINTAIN SAFE WORKING CONDITIONS. SITE SHALL BE SECURED, AS REQUIRED, TO PREVENT UNAUTHORIZED ACCESS TO THE WORK.
4. CARE SHOULD BE TAKEN AT INTERFACE BETWEEN DEMOLITION AND EXISTING CONSTRUCTION TO REMAIN. THIS CARE IS TO AVOID ANY DAMAGE TO EXISTING CONSTRUCTION TO REMAIN, AND TO UTILITIES, WHICH SERVES THAT CONSTRUCTION. THE CONTRACTOR SHALL CORRECT ALL DAMAGE CAUSED BY HIS WORKMEN, AT NO ADDITIONAL COST TO THE OWNER.
5. THE CONTRACTOR SHALL NOTIFY, COORDINATE, SCHEDULE AND RECEIVE PERMISSION FROM THE OWNER PRIOR TO ANY SHUT DOWN OF THE SITE AND/OR BUILDING UTILITIES AS REQUIRED TO COMPLETE THE WORK. NOTIFICATION SHALL INCLUDE THE LENGTH OF TIME REQUIRED TO SHUT DOWN, LENGTH OF TIME SERVICE WILL BE DISCONNECTED, AND TIME REQUIRED TO RECONNECT SERVICES.
6. THE CONTRACTOR SHALL CONFORM TO CITY OF MOBILE REQUIREMENTS FOR THE PROTECTION OF ALL TREES TO REMAIN ON SITE.
7. THE CONTRACTOR SHALL OBTAIN ALL REQUIRED PERMITS BY CITY OF MOBILE, INCLUDING BUT NOT LIMITED TO SIGNAGE AND TREE TRIMMING/REMOVAL PERMITS.
8. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL MEANS AND METHODS, INCLUDING SHORING, BRACING, AND SEQUENCING NECESSARY FOR PROPER COMPLETION OF THE PROJECT.

## STORMWATER EROSION CONTROL NOTE:

THE CONTRACTOR MUST OBTAIN AND SIGN A STORM WATER EROSION CONTROL AGREEMENT WITH THE CITY OF MOBILE. THE CONTRACTOR IS RESPONSIBLE FOR ABIDING BY ADEM REGULATIONS THROUGHOUT THE CONSTRUCTION OF THE PROJECT, AND MUST UNDERSTAND THAT THE CITY WILL ISSUE A STOP WORK ORDER AT ANY TIME THESE MEASURES ARE NOT IN COMPLIANCE UNTIL THE SITE IS IN COMPLIANCE. THE CONTRACTOR SHOULD OBTAIN A COPY OF THESE PRIOR TO BID, SO THAT REQUIREMENTS ARE KNOWN.

## TRAFFIC CONTROL, SAFETY ITEMS:

CONTRACTOR SHALL ERECT ALL WARNING SIGNS, AND PROVIDE THE APPROPRIATE PERSONNEL, IF REQUIRED, AND ALL OTHER ITEMS REQUIRED TO SAFELY HANDLE VEHICULAR AND PEDESTRIAN TRAFFIC THROUGH WORK AREA. CONTRACTOR MUST COORDINATE THIS ACTIVITY WITH THE CITY OF MOBILE TRAFFIC CONTROL DEVICES SHALL BE PROVIDED BY THE CONTRACTOR. TRAFFIC CONTROL DEVICES PROVIDED MUST COMPLY WITH THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES, CURRENT EDITION. CONTRACTOR SHALL UNDERTAKE AND MAINTAIN ADEQUATE SAFETY MEASURES AS AND WHEN NECESSARY TO PROTECT EXISTING ROADS, STREETS, AND WALKWAYS FROM DAMAGE BY VEHICULAR TRAFFIC AND/OR HEAVY EQUIPMENT.

## PROJECT CONSULTANTS:

CIVIL & STRUCTURAL ENGINEER:  
MOTT MACDONALD  
200 W GARDEN ST #700  
PENSACOLA, FL 32502  
850.484.6011

ARCHITECT:  
MOTT MACDONALD  
107 ST FRANCIS ST #2900  
MOBILE, AL 36602  
228.374.1409

MECHANICAL ENGINEER:  
SMITH MECHANICAL  
7150 CHARLANDA CT.  
MOBILE, AL 36695  
251.402.1364

ELECTRICAL CONSULTANT:  
DELL CONSULTING  
813 DOWNTOWN BOULEVARD | SUITE D  
MOBILE, AL 36609  
251.316.0015

## SHEET INDEX

SHEET	DESCRIPTION
D-100	DEMOLITION & EROSION CONTROL PLAN
C3	GRADING & DRAINAGE PLAN
C4	UTILITY PLAN
C5	CIVIL NOTES & DETAILS
E1.0	ELECTRICAL SPECIFICATIONS & ABBREVIATIONS
E2.0	ELECTRICAL LEGEND & NOTES
E3.0	EXISTING ELECTRICAL SITE PLAN
E4.0	NEW WORK SITE PLAN
E5.0	NEW WORK LIGHTING PLAN
E6.0	NEW WORK POWER PLAN
E7.0	ELECTRICAL SCHEDULES & DETAILS
G1.0	ARCHITECTURAL ABBREVIATIONS & STANDARDS
G2.0	BUILDING CODE SUMMARY
G3.0	LIFE SAFETY PLAN
A1.1	ARCHITECTURAL FLOOR PLAN & FINISHES
A1.2	DIMENSIONED FLOOR PLAN
A1.3	CEILING & ROOF FINISH PLAN
A2.1	BUILDING ELEVATIONS
A3.1	BUILDING SECTIONS
A3.2	BUILDING DETAILS
A4.1	BUILDING DETAILS
A4.2	BATHROOM ENLARGEMENT & DETAILS
A5.1	DOOR & WINDOW DETAILS
A5.2	DOOR & WINDOW DETAILS
A6.1	FRAMING DETAILS
A6.2	FRAMING DETAILS
A6.3	FRAMING DETAILS
S0.1	STRUCTURAL SPECIFICATIONS & STANDARDS
S0.2	STRUCTURAL ABBREVIATIONS & STANDARDS
S1.1	FOUNDATION LAYOUT PLAN
S1.2	SLAB ON GRADE LAYOUT PLAN
S1.3	CMU WALL PLAN
S1.4	PERIMETER GIRDER FRAMING PLAN
S1.5	ROOF FRAMING PLAN
S1.6	ROOF FRAMING PLAN
S1.7	STRUCTURAL SECTION ELEVATION PLANS
S3.1	STRUCTURAL DETAILS
S3.2	STRUCTURAL DETAILS
S3.3	STRUCTURAL DETAILS
S3.4	LOAD BEARING WALL FRAMING ELEVATION
S3.5	STRUCTURAL FRAMING DETAILS
P1.0	PLUMBING SCHEDULE & NOTES
P2.0	PLUMBING PLAN
P3.0	SANITARY WASTE PLAN
P4.0	PLUMBING RISER DETAILS
P5.0	PLUMBING DETAILS
M1.0	MECHANICAL NOTES & SCHEDULE
M2.0	HVAC PLAN
M3.0	HVAC DETAILS
H100	HARDSCAPE PLAN
H200	HARDSCAPE DETAILS
L100	LANDSCAPE PLAN

10/17/2024



Know what's below.  
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GENERAL

- 1. TO THE BEST OF OUR KNOWLEDGE, THE STRUCTURAL PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF THE 2021 INTERNATIONAL BUILDING CODE.
2. THE STRUCTURAL DOCUMENTS ARE TO BE USED IN CONJUNCTION WITH THE ARCHITECTURAL, ELECTRICAL, MECHANICAL AND PLUMBING DOCUMENTS. USE THESE NOTES IN CONJUNCTION WITH THE SPECIFICATIONS. IF A CONFLICT EXISTS, THE MORE STRINGENT GOVERNS.
3. COMPLY WITH REQUIREMENTS OF THE BUILDING CODE, OSHA, AND ALL OTHER APPLICABLE FEDERAL, STATE AND LOCAL CODES, STANDARDS, REGULATIONS AND LAWS.
4. ALL REFERENCED STANDARDS REFER TO THE EDITION IN FORCE AT THE TIME THESE PLANS AND SPECIFICATIONS ARE ISSUED FOR PERMITTING.
5. REVIEW ALL CONTRACT DOCUMENTS, DIMENSIONS AND SITE CONDITIONS AND COORDINATE WITH FIELD DIMENSIONS AND PROJECT SHOP DRAWINGS PRIOR TO CONSTRUCTION. REPORT ANY DISCREPANCIES IN WRITING TO ARCHITECT/ENGINEER. DO NOT CHANGE SIZE OR DIMENSIONS OF STRUCTURAL MEMBERS WITHOUT WRITTEN INSTRUCTIONS FROM THE STRUCTURAL ENGINEER OF RECORD.
6. ANY DISCREPANCIES, OMISSIONS OR VARIATIONS NOTED ON THE DRAWINGS OR IN THE SPECIFICATIONS DISCOVERED DURING THE BIDDING PERIOD SHALL BE IMMEDIATELY COMMUNICATED IN WRITING TO THE ARCHITECT/ENGINEER.
7. PROTECT EXISTING FACILITIES, STRUCTURES AND UTILITY LINES FROM ALL DAMAGE. EACH CONTRACTOR SHALL PROTECT HIS WORK, ADJACENT PROPERTY AND THE PUBLIC. EACH CONTRACTOR IS SOLELY RESPONSIBLE FOR DAMAGE OR INJURY DUE TO HIS ACT OR NEGLIGENCE.
8. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR JOB SAFETY AND CONSTRUCTION PROCEDURES.
9. DO NOT SCALE DRAWINGS; USE DIMENSIONS.
10. REFER TO ARCHITECTURAL, ELECTRICAL, MECHANICAL AND PLUMBING DRAWINGS FOR SIZE AND LOCATION OF OPENINGS IN STRUCTURE NOT SHOWN ON STRUCTURAL DRAWINGS.
11. DETAILS LABELED "TYPICAL DETAILS" OR "TYP" ON THE DRAWINGS APPLY TO ALL SITUATIONS THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY DETAILED. SUCH DETAILS APPLY WHETHER OR NOT THEY ARE KEYED IN AT EACH LOCATION. QUESTIONS REGARDING APPLICABILITY OF TYPICAL DETAILS SHALL BE RESOLVED BY THE ARCHITECT/ENGINEER.
12. DESIGN LOADS AND CRITERIA :
A. LIVE LOADS :
- ROOF: 20 PSF
- FLOOR: 100 PSF
- CONCENTRATED LOADS : AS SHOWN ON PLANS
B. TRUSS DESIGN LOADS
- TOP CHORD DEAD LOAD MAX: 15 PSF
- TOP CHORD DEAD LOAD MIN: 5 PSF
- BOTTOM CHORD DEAD LOAD MAX: 10 PSF
- BOTTOM CHORD DEAD LOAD MIN: 5 PSF
- ROOF LIVE LOAD: 20 PSF
- ROOF UPLIFT PRESSURES: REFER TO S1.4
C. WIND CRITERIA (ASCE 7-16):
- ADDRESS: 5400 GRISHILDE DRIVE MOBILE, ALABAMA
- WIND SPEED, V ult: 160 MPH
- EXPOSURE : B
- RISK CATEGORY : II
- ENCLOSURE: ENCLOSED\*
\* TO ACHIEVE ENCLOSED CLASSIFICATION, ALL GLAZED OPENINGS SHALL BE IMPACT RESISTANT OR PROTECTED WITH IMPACT RESISTING COVERING. ALL LOUVERS FOR THE FIRST 30 FEET SHALL MEET THE REQUIREMENTS OF AN APPROVED IMPACT-RESISTING STANDARD OF THE LARGE MISSILE TEST OF ASTM E1996.
D. SEISMIC CRITERIA (ASCE 7-16) :
- IMPORTANCE FACTOR : 1.00
- MAPPED RESPONSE ACCELERATION, S2: 0.130
- MAPPED RESPONSE ACCELERATION, S1: 0.068
- RESPONSE COEFFICIENT, Sps1: 0.097
- RESPONSE COEFFICIENT, Sps2: 0.11
- DESIGN CATEGORY : B
- SEISMIC-FORCE-RESISTING SYSTEM : LIGHT-FRAME SHEATHED BEARING WALLS
- RESPONSE MODIFICATION FACTOR, R : 7.0
- RESPONSE COEFFICIENT, Cs: 0.014
- ANALYSIS PROCEDURE : EQUIVALENT LATERAL FORCE PROCEDURE
E. SNOW LOAD CRITERIA (ASCE 7-16) :
- GROUND SNOW LOAD : 0 PSF
F. REFERENCE DATA AND FLOOD DATA:
- ADDRESS: 5400 GRISHILDE DRIVE MOBILE, ALABAMA
- STRUCTURAL PLANS ARE BASED ON TOP OF CONCRETE FOR THE GROUND LEVEL 0'-0" = 97.00' NGVD (COORDINATE WITH CIVIL DRAWINGS)
- PROJECT LOCATED IN FLOOD ZONE X.

SHOP DRAWING SUBMITTAL

- 1. THE FOLLOWING REQUIREMENTS IN NO WAY REDUCE OR LIMIT ANY ADDITIONAL REQUIREMENTS OF THE SPECIFICATIONS.
2. REVIEW OF SUBMITTALS BY THE STRUCTURAL ENGINEER IS FOR GENERAL COMPLIANCE WITH THE CONTRACT DOCUMENTS. RESPONSIBILITY FOR THE CORRECTNESS OF DIMENSIONS, DETAILS, QUANTITIES, AND SAFETY DURING FABRICATION AND CONSTRUCTION SHALL REMAIN WITH THE CONTRACTOR.
3. CORRECTIONS AND/OR COMMENTS MADE ON THE SHOP DRAWINGS DURING REVIEW DO NOT IMPLY THAT ALL ERRORS AND OMISSIONS HAVE BEEN CORRECTED, NOR DOES IT RELIEVE THE CONTRACTOR FROM COMPLIANCE WITH THE CONTRACT DOCUMENTS.
4. NO DETAILED CHECK OF QUANTITIES OR DIMENSIONS WILL BE MADE. ONLY THOSE SHOP DRAWINGS REQUIRED BY THE CONTRACT DOCUMENTS TO BE SUBMITTED WILL BE REVIEWED. ALL OTHERS WILL BE RETURNED WITHOUT COMMENT.
5. SHOP DRAWINGS WILL NOT BE REVIEWED UNLESS THEY ARE STAMPED "APPROVED" OR "APPROVED AS NOTED" BY THE GENERAL CONTRACTOR OR CONSTRUCTION MANAGER, WHICH EVER IS APPLICABLE.
6. THE CONTRACTOR SHALL INFORM THE ENGINEER IN WRITING OF ANY SPECIFIC DEVIATIONS TO THE CONTRACT DOCUMENTS AND OBTAIN ENGINEER'S WRITTEN APPROVAL BEFORE PROCEEDING.
7. IN ACCORDANCE WITH THE SPECIFICATIONS SUBMIT SHOP DRAWINGS CONSISTENT WITH THE FOLLOWING CRITERIA:
A. ALLOW ADEQUATE TIME FOR TRANSIT AND PROCESSING BEFORE FABRICATION.
B. SCHEDULE AND SUBMIT SHOP DRAWINGS FOR SPECIFIC COMPONENTS, SUCH AS COLUMNS, FOOTINGS, ETC., IN THEIR ENTIRETY. SHOP DRAWINGS FOR SIMILAR FLOORS SHALL BE SUBMITTED IN THE SAME PACKAGE.
C. SUBMIT SHOP DRAWINGS IN A TIMELY MANNER, CONSISTENT WITH THE ABOVE REQUIREMENTS.
8. ALL CHANGES AND ADDITIONS MADE ON RESUBMITTALS MUST BE CLEARLY FLAGGED AND NOTED. THE PURPOSE OF THE RESUBMITTALS MUST BE CLEARLY NOTED ON THE LETTER OF TRANSMITTAL. ARCHITECT / ENGINEER REVIEW WILL BE LIMITED TO THE ITEMS CAUSING THE RESUBMITTAL.

- 9. DO NOT REPRODUCE THE CONTRACT DOCUMENTS FOR USE AS SHOP DRAWINGS.
10. SHOP DRAWINGS NOT MEETING THE ABOVE CRITERIA OR SUBMITTED AFTER FABRICATION WILL NOT BE REVIEWED.
11. RESPONSIBILITIES OF DETAILERS AND FABRICATORS:
A. GENERAL- SUBMIT SHOP DRAWINGS AND ANY OTHER SPECIAL INFORMATION NECESSARY FOR PROPER FABRICATION, ERECTION, AND PLACEMENT OF STRUCTURAL FABRICATIONS, INCLUDE PLANS, ELEVATIONS, AND SECTIONS. CLEARLY SHOW ANCHORAGES, CONNECTIONS, AND ACCESSORY ITEMS. THE DETAILER MUST INTERPRET THE CONTRACT DOCUMENTS AND CLEARLY CONVEY THIS INTERPRETATION TO THE FIELD IN THE FORM OF PLACING OR ERECTION DRAWINGS.
B. CONCRETE REINFORCING DETAILER- PROVIDE PLACING DRAWINGS FOR FABRICATION AND PLACING OF REINFORCING STEEL. THESE DRAWINGS SHALL INCLUDE, BUT ARE NOT LIMITED TO THE FOLLOWING: BAR LISTS, SCHEDULES, BENDING DETAILS, PLACING DETAILS, PLACING PLANS, AND PLACING ELEVATIONS.
C. CLEARLY SHOW ELEVATION, SECTIONS, AND DETAILS OF ALL BEAM TO COLUMN CONNECTIONS.
D. CLEARLY SHOW COLUMN ELEVATIONS AND SECTIONS. INDICATE DOWELS, OFFSETS, LAP SPLICES, AND TIES. PLAN SECTIONS OF ALL COLUMNS MUST CLEARLY BE SHOWN.
E. CLEARLY SHOW ELEVATIONS OF ALL BEARING AND SHEAR WALLS. INDICATE OPENINGS, DETAILS OF ALL REINFORCING WITH LOCATIONS OF SPLICES AND HOOKS, ALL CONTROL JOINTS, EXPANSION JOINTS, LINTELS, CONCRETE BOND BEAMS, AND PILASTERS. CLEARLY SHOW BEAM ELEVATIONS AND SECTIONS. INDICATE BAR LENGTHS, HOOKS, STIRRUP SPACING, LAP SPLICES, OFFSETS, AND LOCATION OF BARS WITH RESPECT TO ALL SUPPORTS.
F. CLEARLY SHOW FOUNDATION REINFORCING. INDICATE BAR LENGTHS, LOCATION AND SPLICES OF CONTINUOUS BARS, AND BAR SUPPORTS. CLEARLY SHOW LOCATIONS OF ALL DOWELS ON PLAN. INDICATE FOOTING STEP LOCATIONS AND PROVIDE DETAILS.
12. FOR ADDITIONAL CRITERIA APPLICABLE TO SHOP DRAWINGS REQUIRING ENGINEERING INPUT BY A SPECIALTY ENGINEER, REFER TO "SHOP DRAWING REQUIRING ENGINEERING INPUT BY SPECIALTY ENGINEER" GENERAL NOTE SECTION.

SHOP DRAWING REQUIRING ENGINEERING INPUT BY SPECIALTY ENGINEER

- 1. SPECIALTY ENGINEER:
A. DEFINITION - A REGISTERED PROFESSIONAL ENGINEER IN THE STATE THE PROJECT IS LOCATED WHO SPECIALIZES IN AND WHO UNDERTAKES THE DESIGN OF STRUCTURAL COMPONENTS OR STRUCTURAL SYSTEMS INCLUDED IN A SPECIFIC SUBMITTAL PREPARED FOR THIS PROJECT.
B. SHALL BE:
A. AN EMPLOYEE OR OFFICER OF A FABRICATOR.
B. AN EMPLOYEE OR OFFICER OF AN ENTITY SUPPLYING COMPONENTS TO A FABRICATOR.
C. AN INDEPENDENT CONSULTANT RETAINED BY THE FABRICATOR OR HIS SUPPLIER.
2. THE FOLLOWING SYSTEMS AND COMPONENTS AS A MINIMUM REQUIRE FABRICATION AND ERECTION DRAWINGS WITH INPUT BY A SPECIALTY ENGINEER, BUT ARE NOT LIMITED TO: TRUSSES, GIRDER TRUSSES, SPECIAL ENGINEERED WOOD, SHORING AND RESHORING, WINDOWS, STOREFRONT, CURTAIN WALL SYSTEMS, DOORS, ROOF SYSTEMS, PRE-ENGINEERED STAIRS, LOUVERS, SIDING AND ANY EXTERIOR ANCILLARY STRUCTURES.
3. THE SPECIALTY ENGINEER OR MANUFACTURER SHALL DESIGN, PROVIDE, AND INSTALL THEIR COMPONENTS AND THE COMPONENT CONNECTIONS TO THE PRIMARY STRUCTURE PER THE WIND CRITERIA STATED IN THESE NOTES OR THE CURRENT GOVERNING BUILDING CODES, WHICHEVER IS MORE STRINGENT.
4. SUBMITTALS SHALL CLEARLY IDENTIFY THE SPECIFIC PROJECT AND APPLICABLE CODES, LIST THE DESIGN CRITERIA, AND SHOW ALL DETAILS AND PLANS NECESSARY FOR PROPER FABRICATION AND INSTALLATION. CALCULATIONS AND SHOP DRAWINGS SHALL IDENTIFY SPECIFIC PRODUCT UTILIZED. GENERIC PRODUCTS WILL NOT BE ACCEPTED.
5. SHOP DRAWINGS AND CALCULATIONS MUST BE PREPARED UNDER THE DIRECT SUPERVISION AND CONTROL OF THE SPECIALTY ENGINEER.
6. SHOP DRAWINGS AND CALCULATIONS REQUIRE THE SEAL, DATE AND SIGNATURE OF THE SPECIALTY ENGINEER. COMPUTER PRINTOUTS ARE AN ACCEPTABLE SUBSTITUTE FOR MANUAL COMPUTATIONS PROVIDED THEY ARE ACCOMPANIED BY SUFFICIENT DESCRIPTIVE INFORMATION TO PERMIT THEIR PROPER EVALUATION. SUCH DESCRIPTIVE INFORMATION SHALL BEAR THE EMBOSSED SEAL AND SIGNATURE OF THE SPECIALTY ENGINEER AS AN INDICATION THAT HE HAS ACCEPTED RESPONSIBILITY FOR THE RESULTS. THE STRUCTURAL ENGINEER WILL RETAIN ONE SIGNED AND SEALED COPY FOR RECORD.
7. CATALOG INFORMATION ON STANDARD PRODUCTS DOES NOT REQUIRE THE SEAL OF A SPECIALTY ENGINEER.
8. REVIEW BY THE STRUCTURAL ENGINEER OF RECORD OF SUBMITTALS IS LIMITED TO VERIFYING THE FOLLOWING:
A. THAT THE SPECIFIED STRUCTURAL SUBMITTALS HAVE BEEN FURNISHED.
B. THAT THE STRUCTURAL SUBMITTALS HAVE BEEN SIGNED AND SEALED BY THE SPECIALTY ENGINEER.
C. THAT THE SPECIALTY ENGINEER HAS UNDERSTOOD THE DESIGN INTENT AND HAS USED THE SPECIFIED STRUCTURAL CRITERIA, (NO DETAILED CHECK OF CALCULATIONS WILL BE MADE.)
D. THAT THE CONFIGURATION SET FORTH IN THE STRUCTURAL SUBMITTALS IS CONSISTENT WITH THE CONTRACT DOCUMENTS, (NO DETAILED CHECK OF DIMENSIONS OR QUANTITIES WILL BE MADE.)

SOIL PREPARATION, SOIL COMPACTION, AND GEOTECHNICAL CONSIDERATIONS FOR FOOTING DESIGN

- 1. GEOTECHNICAL REPORT - GEOTECHNICAL ENGINEERING TESTING, INC. (GET) - REPORT DATE: 12/22/2023 REPORT NAME: SOILS EXPLORATIONS AND GEOTECHNICAL ENGINEERING STUDIES FOR BUILDING ADDITION AT MIMMS PARK, MOBILE ALABAMA.
2. BASIS OF DESIGN CONSIDERS 1500 PSF NET ALLOWABLE BEARING PRESSURE.
3. SOIL COMPACTION BENEATH THE SPREAD FOOTINGS SHALL BE 100% STANDARD PROCTOR. SOIL COMPACTION SHALL BE MONITORED BY A GEOTECHNICAL ENGINEER LICENSED IN THE STATE OF ALABAMA.
4. EXCAVATE EXISTING SOIL TO BOTTOM OF FOOTINGS. ALL DELETERIOUS MATERIAL MUST BE COMPLETELY REMOVED.
5. ALL EXISTING UTILITIES & ORGANICS (INCLUDING STUMPS AND ROOTS) SHALL BE COMPLETELY REMOVED PRIOR TO FILL OPERATIONS.
6. SOIL COMPACTION, FILL, AND ITS REPLACEMENT SHALL BE FIELD CONTROLLED BY THE TESTING AGENCY OR GEOTECHNICAL ENGINEER OF RECORD. THE TESTING AGENCY SHALL RANDOMLY SELECT ALL TEST LOCATIONS.
7. THE CONTRACTOR SHALL DETERMINE WHETHER DE-WATERING WILL BE REQUIRED BASED ON ACTUAL GROUND WATER CONDITIONS AT THE TIME OF CONSTRUCTION.

SLABS ON GRADE

- 1. GEOTECHNICAL REPORT - GEOTECHNICAL ENGINEERING TESTING, INC. (GET) - REPORT DATE: 12/22/2023 REPORT NAME: SOILS EXPLORATIONS AND GEOTECHNICAL ENGINEERING STUDIES FOR BUILDING ADDITION AT MIMMS PARK, MOBILE ALABAMA.
2. SOIL COMPACTION BENEATH THE SLABS ON GRADE SHALL BE 100% STANDARD PROCTOR. SOIL COMPACTION SHALL BE MONITORED BY A GEOTECHNICAL ENGINEER LICENSED IN THE STATE OF ALABAMA.
3. USE 15 MIL. POLYETHYLENE SHEETING BETWEEN SOIL AND CONCRETE SLAB, UNLESS OTHERWISE NOTED.
4. RECESS SLABS ON GRADE FOR FLOOR FINISHES PER ARCHITECTURAL DRAWINGS.
5. THE SLAB ON GRADE SHALL BE SUPPORTED BY STRUCTURAL FILL MATERIAL AND 4-INCHES OF FREE DRAINING GRANULAR SOILS OR GRAVEL MEETING ALDOT CRITERIA BENEATH THE POLYETHYLENE SHEETING FOR SUPPORT THE SLAB ON GRADE.
6. REFER TO PLAN FOR THICKNESS AND DIMENSIONS.

PLAIN AND REINFORCED CONCRETE

- 1. USE STRUCTURAL CONCRETE AND CONCRETING PRACTICES CONFORMING TO ACI-316 AND 301 AND PROPORTION CONCRETE IN ACCORDANCE WITH ACI-318 CH. 4 AND MEETING A MINIMUM ULTIMATE COMPRESSIVE STRENGTH IN 28 DAYS AS FOLLOWS:

Table with 2 columns: SLABS AND FOOTINGS (4000 PSI), ALL OTHER CONCRETE (4000 PSI), PROVIDE CURRENT (MAXIMUM, 1 YEAR OLD) STATISTICAL DATA FOR EACH CONCRETE MIX DESIGN SUBMITTED.

MOTT MACDONALD Architects Engineers Surveyors 107 St. Francis Street Suite 2900, Mobile, Alabama 36602 Telephone: (251) 343-4368 Fax: (251) 343-6902

- 2. WHERE CONCENTRATION OF REINFORCING STEEL HINDERS PROPER CONSOLIDATION OF CONCRETE USE CONCRETE CONTAINING A SUPERPLASTICIZER (N.R.W.R.) ADMIXTURE, ASTM C494 TYPE F. SLUMP AFTER ADDITION OF SUPERPLASTICIZER SHALL BE 7" ±1".
3. IF CONCRETE IS PUMPED, SLUMP MAY BE INCREASED TO 6" AT THE TRUCK. USE A MINIMUM 4-INCH PUMP, UNLESS PRE-APPROVED BY ENGINEER. TAKE CONCRETE SAMPLES FOR SLUMP AT TRUCK AND AT DISCHARGE END. TAKE CONCRETE SAMPLES FOR CYLINDER TESTING AT DISCHARGE END.
4. PROVIDE CONSTRUCTION JOINTS IN ACCORDANCE WITH ACI 318 CH. 6.4 AND SUBMIT SHOP DRAWINGS SHOWING LOCATIONS AND DIRECTION OF CONCRETE PLACEMENT FOR STRUCTURAL ENGINEER'S REVIEW. ROUGHEN JOINTS AND EXTEND ALL REINFORCEMENT THROUGH JOINT. PROVIDE CLASS B LAP SPlice BEYOND JOINT. PROVIDE JOINTS IN MIDDLE THIRD OF ALL SLAB & BEAMS U.O.N.
7. PROVIDE REINFORCING STEEL ERECTOR WITH A SET OF STRUCTURAL PLANS FOR FIELD USE. INSPECT REINFORCING STEEL PLACING FROM STRUCTURAL PLANS.
8. USE ASTM A-615 GR. 60 FOR ALL REINFORCING STEEL. CONFORM TO ACI-301, ACI-315, ACI-318, AND CRSI "MANUAL OF STANDARD PRACTICE". ALL REINFORCING SHALL BE ACCURATELY PLACED, RIGIDLY SUPPORTED AND FIRMLY TIED IN PLACE WITH BAR SUPPORTS AND SPACERS IN ACCORDANCE WITH THE ABOVE REQUIREMENTS. PROVIDE CLASS 'B' LAP SPlice FOR CONTINUOUS BARS, UNLESS OTHERWISE NOTED. LAP BOTTOM STEEL OVER SUPPORTS AND TOP STEEL AT MID SPAN UNLESS OTHERWISE SPECIFIED. HOOK DISCONTINUOUS ENDS OF ALL TOP BARS AND ALL BARS IN WALLS, UNLESS OTHERWISE NOTED.
9. PLACE REINFORCING STEEL SUCH THAT BARS ADJACENT TO CONCRETE SURFACES & COLD JOINTS MEET MINIMUM CLEAR COVER REQUIREMENTS, BUT DO NOT EXCEED THOSE REQUIREMENTS. USE THE FOLLOWING CLEAR COVER OVER REINFORCING:

Table with 3 columns: SLABS, FOOTINGS AND RETAINING WALLS, FOOTINGS, PIERS. Rows for BOTTOM, TOP, SIDES with dimensions like 3", 2", 1 1/2", 1 1/2".

- 10. USE PLAIN, COLD-DRAWN ELECTRICALLY-WELDED STEEL WIRE FABRIC CONFORMING TO ASTM A-185. SUPPLY IN FLAT SHEETS ONLY. LAP SPLICES SHALL BE MEASURED BETWEEN OUTERMOST CROSS WIRES OF EACH FABRIC SHEET AND SHALL BE NOT LESS THAN TWICE THE SPACING OF THE CROSS WIRES PLUS TWO (2) INCHES.
11. SLEEVE ALL PIPES THROUGH SLABS INDIVIDUALLY, UNLESS APPROVED BY ENGINEER.
12. SUBMIT SHOP DRAWINGS TO THE ENGINEER FOR REVIEW PRIOR TO FABRICATING REINFORCING STEEL. DO NOT REPRODUCE THE STRUCTURAL DRAWINGS FOR USE AS SHOP DRAWINGS.
13. PROVIDE CLASS 'B' LAP SPlice AT SUPPORTS AND HOOK DISCONTINUOUS ENDS AT THE FAR FACE OF SUPPORTS FOR ALL BEAMS, UNLESS OTHERWISE NOTED.
14. REINFORCING PLACED IN LOCATIONS WHERE PROPER COVER CANNOT BE ACHIEVED SHALL BE HOT DIPPED GALVANIZED ACCORDING TO ASTM A767 WITH 2 OUNCES OF ZINC COATING PER SQUARE FOOT OF SURFACE AREA MINIMUM.
15. ALL EXPOSED CONCRETE AND GROUT EDGES SHALL HAVE 3/4", 45° CHAMFER UNLESS OTHERWISE NOTED.
WOOD
1. DESIGN AND CONSTRUCT IN ACCORDANCE WITH THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION AND THE AMERICAN PLYWOOD ASSOCIATION.
2. WOOD SHALL BE PRESSURE TREATED NO. 1 SOUTHERN PINE OR BETTER MATCHING THE SIZES SHOWN ON THE PLANS, TYPICAL UNLESS OTHERWISE NOTED.
3. ALL 2x6 WOOD STUDS SHALL BE #1 SOUTHERN YELLOW PINE.
4. WALL TOP PLATE: DOUBLE 2x6 TOP PLATE AND A BEVELED 4x6 TOP PLATE CUT TO MATCH THE ROOF SLOPE. NAIL HEADER ASSEMBLY TOGETHER WITH (3) GALV. 20d NAILS @ 20" ON CENTER.
5. USE INTERNATIONAL BUILDING CODE APPROVED NAILING SCHEDULES WHERE NAILING INFORMATION IS NOT PROVIDED.
6. REFER TO STEEL FASTENERS/HARDWARE AND PRESERVATIVE TREATMENT NOTES FOR ADDITIONAL INFORMATION.
7. PLYWOOD WALL & ROOF SHEATHING SHALL BE APA STRUCTURAL 1 RATED SHEATHING EXTERIOR, MATCHING THE THICKNESS SHOWN ON THE PLANS.
8. LOW ROOF JOISTS SECTION CRITERIA SHALL MEET THE FOLLOWING:

- A. MINIMUM TOTAL DEPTH: 11 7/8"
B. MINIMUM TOP AND BOTTOM FLANGE WIDTHS: 2"
C. MINIMUM WEB THICKNESS: 3/8"
D. MINIMUM FLANGE THICKNESS: 1.5"
E. MINIMUM LOAD CAPACITY: 134 PLF
F. MINIMUM DEFLECTION CRITERIA: L/240 <1.0"

- 9. UPPER ROOF JOISTS SECTION CRITERIA SHALL MEET THE FOLLOWING:
A. MINIMUM TOTAL DEPTH: 11 7/8"
B. MINIMUM TOP AND BOTTOM FLANGE WIDTHS: 1.75"
C. MINIMUM WEB THICKNESS: 3/8"
D. MINIMUM FLANGE THICKNESS: 1 1/8"
E. MINIMUM LOAD CAPACITY: 225 PLF
F. MINIMUM DEFLECTION CRITERIA: L/240 <0.5"

- 10. ALL LVL MEMBERS SECTION CRITERIA SHALL MEET THE FOLLOWING:

Table with 7 columns: LOCATION, MEMBER DESIGNATION, MEMBER DEPTH, MIN. SHEAR CAPACITY, MIN. MOMENT CAPACITY, WT. PER FT, Ixx. Rows for WALL HEADERS, PORCH BEAMS, LOW ROOF, UPPER ROOF.

PRESERVATIVE TREATMENT

- 1. TO THE EXTENT POSSIBLE, ALL WOOD SHALL BE CUT, DRILLED, AND COMPLETELY FABRICATED PRIOR TO PRESSURE TREATMENT. WHEN FIELD FABRICATION OF WOOD IS REQUIRED OR IF WOOD IS DAMAGED, ALL CUTS, BORE HOLES, AND DAMAGE SHALL BE IMMEDIATELY FIELD TREATED WITH WOOD PRESERVATIVE IN ACCORDANCE WITH AWPA STANDARDS.
2. ALL LUMBER SHALL BE TREATED IN ACCORDANCE WITH THE REQUIREMENT OF AWPA STANDARDS AND ASTM D1760.
3. TREATED MATERIAL SHALL BE FREE OF EXCESS PRESERVATIVE ON THE WOOD SURFACE. THE TREATING PROCESS SHALL INCLUDE AN EXPANSION BATH, STEAMING AND/OR DRIPPING TO ENSURE THAT PRESERVATIVE WILL NOT BLEED.
4. TREATED WOOD SHALL BE INSPECTED AND CERTIFIED IN ACCORDANCE WITH AWPA STANDARDS.
5. WOOD ATTACHED DIRECTLY TO CONCRETE OR MASONRY OR EXPOSED TO THE WEATHER SHALL BE PRESSURE TREATED WITH ALKALINE COPPER QUAT - TYPE C (ACQ-C) AT THE RATE OF 0.60 LBS/ CU FT.

CHRISTIANPREUS Landscape Architecture ARCHITECTURAL DRAWINGS FOR: CITY OF MOBILE - MIMS PARK CONCESSIONS ARCHITECT: DATE: May 5, 2024 SCALE: 1/2" = 1'-0" ISSUED FOR PERMIT No. 28305 PROFESSIONAL ENGINEER 04-24-2024 CHD EDWARD LANK

STEEL FASTENERS AND HARDWARE

- 1. FASTENERS IN CONTACT WITH ACQ TREATED LUMBER SHALL EITHER BE TYPE 316 STAINLESS STEEL OR SHALL BE HOT DIPPED GALVANIZED ACCORDING TO ASTM A153 WITH 2 OUNCES OF ZINC COATING PER SQUARE FOOT MINIMUM, AND SEPARATED WITH A WATERPROOF MEMBRANE.
2. FASTENERS AND CONNECTORS USED TOGETHER SHOULD BE OF THE SAME TYPE. DO NOT MIX HOT-DIPPED GALVANIZED ITEMS WITH STAINLESS STEEL.
3. ALL INDOOR VISUALLY EXPOSED CONNECTORS AND FASTENERS SHALL BE 316 STAINLESS STEEL.
4. ALL STEEL PLATES AND SHAPES SHALL COMPLY WITH THE REQUIREMENTS OF 316 STAINLESS STEEL.
5. BOLTS AND LAG SCREWS SHALL COMPLY WITH THE REQUIREMENTS OF 316 STAINLESS STEEL, AND SHALL PREFERABLY BE DOME HEAD TIMBER BOLTS.
6. WASHERS & NUTS SHALL BE PROVIDED UNDER BOLT AND LAG SCREW HEADS AND NUTS THAT ARE IN CONTACT WITH WOOD AND SHALL BE HOT DIPPED GALVANIZED OR 316 STAINLESS STEEL. WASHERS MAY BE OMITTED UNDER HEADS OF SPECIAL TIMBER BOLTS OR DOME HEAD BOLTS WHEN THE SIZE AND STRENGTH OF THE HEAD IS SUFFICIENT TO DEVELOP CONNECTION STRENGTH WITHOUT WOOD CRUSHING.
7. ALL SIMPSON CONNECTORS SHALL BE GALVANIZED.
8. CONNECTORS SHALL BE SIMPSON OR ENGINEERED APPROVED EQUAL. CONNECTORS CALLED OUT ON PLANS ARE SIMPSON MODEL NUMBERS. INSTALL CONNECTORS ACCORDING TO THE MANUFACTURER'S WRITTEN INSTRUCTIONS USING THE MAXIMUM NUMBER OF CONNECTORS, UNLESS OTHERWISE NOTED.
9. ALL NAILS, BOLTS, AND CONNECTORS EXPOSED TO THE WEATHER OR IN CONTACT WITH TREATED LUMBER SHALL BE GALVANIZED OR STAINLESS STEEL. USE STANDARD CODE APPROVED NAILING SCHEDULES WHERE NAILING INFORMATION IS NOT PROVIDED. EXTERIOR NAILS PLACED IN TREATED LUMBER SHALL BE RING, SHANKED.
A. TYPE 316 STAINLESS STEEL SHALL BE USED FOR ALL FASTENERS AND CONNECTORS EXPOSED TO OCEAN SALT AIR.
B. FASTENERS IN CONTACT WITH ACQ TREATED LUMBER AND NOT EXPOSED TO OCEAN SALT AIR SHALL EITHER TYPE 316 STAINLESS STEEL OR SHALL BE HOT-DIP GALVANIZED ACCORDING TO ASTM A153 WITH 2 OUNCES OF ZINC COATING PER SQUARE FOOT MINIMUM.
C. CONNECTORS IN CONTACT WITH ACQ TREATED LUMBER AND NOT EXPOSED TO OCEAN SALT AIR SHALL EITHER TYPE 316 STAINLESS STEEL OR SHALL BE HOT-DIP GALVANIZED ACCORDING TO ASTM A653, CLASS G185 SHEET WITH 1.85 OUNCES OF ZINC COATING PER SQUARE FOOT MINIMUM. GALVANIZED CONNECTORS SHALL BE SEPARATED FROM ACQ TREATED LUMBER USING A PHYSICAL SPACER/BARRIER MATERIAL SUCH AS GRACE VYCOR DECK PROTECTOR OR BY GIVING THE CONTACT SURFACE A HEAVY COAT OF ALKALI RESISTANT BITUMINOUS PAINT.
D. FASTENERS AND CONNECTERS USED TOGETHER SHOULD BE OF THE SAME TYPE; DO NOT MIX HOT-DIP GALVANIZED ITEMS WITH STAINLESS STEEL.
E. ALUMINUM PRODUCTS SHALL BE SEPARATED FROM WOOD, INCLUDING ACT TREATED LUMBER, USING A PHYSICAL SPACER/BARRIER MATERIAL SUCH AS GRACE VYCOR DECK PROTECTOR OR BY GIVING THE CONTACT SURFACE A HEAVY COAT OF ALKALI RESISTANT BITUMINOUS PAINT.
F. STAINLESS STEEL FASTENERS AND CONNECTORS IN CONTACT WITH ALUMINUM IN THE PRESENCE OF OCEAN SALT AIR SHALL BE PAINTED PRIOR TO INSTALLATION, COAT ALUMINUM WITH CHROMATE CONVERSION COATING OR SEPERATE STAINLESS WASHERS FROM ALUMINUM WITHA NEOPRENE WASHER.

ANCHORS & POST INSTALLED REINFORCING

- 1. SUBSTITUTION OF ANCHORS SPECIFIED BELOW FOR CAST-IN-PLACE EMBEDDED ANCHORS SHALL BE PROHIBITED WITHOUT PRIOR WRITTEN APPROVAL FROM TEH ENGINEER OF RECORD.
2. ALLOWABLE WORKING LOADS SHALL NOT EXCEED MANUFACTURER'S RECOMMENDATIONS, BUT NOT MORE THAN ACCEPTED BY APPROVING AGENCY. NO INCREASE FOR WIND OR SEISMIC LOADS IS PERMITTED.
3. PROVIDE A MINIMUM OF TWO FASTENERS PER CONNECTION.
4. INSTALL AND MAINTAIN A MINIMUM EMBEDMENT IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS, OR AS SPECIFIED ON DRAWINGS, WHICHEVER IS GREATER, BUT WITH AN EMBEDMENT OF NOT LESS THAN 6 BOLT DIAMETERS.
5. UNLESS NOTED, ANCHOR SPACING AND ANCHOR EDGE DISTANCE SHALL BE ACCORDING TO THE MANUFACTURER'S MOST CURRENT PUBLICATION IN ORDER TO DEVELOP MAXIMUM WORKING LOADS.
6. DO NOT EXCEED MANUFACTURER'S MAXIMUM RECOMMENDED TIGHTENING TORQUE.
7. ALL ANCHORS SHALL BE INSTALLED AS PER MANUFACTURER'S RECOMMENDATIONS AND UNDER MANUFACTURER CERTIFIED SUPERVISION IN ORDER TO DEVELOP THE MOST CURRENT PUBLISHED WORKING LOADS.
8. EXPANSION ANCHORS: USE 316 STAINLESS STEEL WEDGE-TYPE EXPANSION ANCHORS SUCH AS HILTI KWIK BOLT II OR ENGINEERED APPROVED EQUIVALENT.
9. ALL DRILLED AND EPOXIED ANCHORS, THREADED RODS OR BOLTS SHALL BE 316 STAINLESS STEEL.
10. ADHESIVE ANCHORING SYSTEMS FOR 316 STAINLESS STEEL ANCHORS, THREADED RODS OR BOLTS:
A. USE AN EPOXY OR POLYESTER RESIN ADHESIVE SUCH AS HILTI RE 500, SIMPSON SET OR ACCEPTED ALTERNATE.
B. DIAMETER OF HOLE SHALL BE AS RECOMMENDED BY MANUFACTURE FOR THE PARTICULAR PRODUCT SPECIFIED IN THE DRAWINGS.
C. ALL EPOXIED ANCHORING SHALL BE OBSERVED BY A MANUFACTURER'S AUTHORIZED REPRESENTATIVE OR SHALL BE TESTED AFTER INSTALLATION AT CONTRACTOR'S EXPENSE. A MINIMUM OF 10% OF EACH DAY'S APPLICATIONS AND NO LESS THAN 2 SHALL BE TESTED BY THE FOLLOWING:
1. REINFORCING STEEL: APPLY A TENSION LOAD OF 3000 lb TO THE EMBEDDED ANCHOR
2. THREADED RODS AND BOLTS: APPLY 50% OF MAXIMUM ALLOWABLE TORQUE AS RECOMMENDED BY MANUFACTURER
IF A TEST APPLICATION FAILS, ALL APPLICATIONS FOR THAT DAY SHALL BE TESTED. TESTING PROCEDURES AND RESULTS SHALL BE SUBMITTED AND APPROVED BY ENGINEER.
10. POWDER ACTUATED FASTENERS: USE POWDER ACTUATED FASTENING SYSTEMS SUCH AS HILTI, RED HEAD, RAMSET, OR AN ACCEPTED ALTERNATE HAVING ICBO, OR SBCCI APPROVAL. INSTALL IN ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS, BUT NOT LESS THAN 1 1/8" INCHES IN CONCRETE, UNLESS OTHERWISE NOTED.
11. ANCHOR INSTALLATION SHALL ENSURE RECOMMENDED MANUFACTURER LOADS CAN BE ACHIEVED.

CONCRETE MASONRY UNITS:

- 1. ALL MASONRY DESIGN SHALL CONFORM TO TMS 402/602.
2. REINFORCED MASONRY WALL DESIGN IS BASED ON INSPECTED MASONRY AS REQUIRED BY TMS 402/602 SPECIFICATION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING A QUALITY CONTROL AND INSPECTION PROGRAM TO INSURE THAT ALL MASONRY WALL CONSTRUCTION IS IN COMPLIANCE WITH THE CONTRACT DOCUMENTS. REFER TO SPECIFICATION FOR THE MINIMUM REQUIREMENTS FOR THIS PROGRAM.
3. ALL MASONRY CONSTRUCTION AND MATERIALS SHALL CONFORM TO ALL REQUIREMENTS OF "BUILDING CODE REQUIREMENTS AND SPECIFICATIONS FOR MASONRY STRUCTURES (TMS 402/602)" PUBLISHED BY THE MASONRY SOCIETY, EXCEPT AS MODIFIED BY THE REQUIREMENTS OF THE CONTRACT DOCUMENTS.
4. CONSTRUCT REINFORCED AND UNREINFORCED MASONRY AS NOTED ON THE PLANS AND DETAILS AND IN ACCORDANCE WITH THE REQUIREMENTS OF THE "SPECIFICATION FOR MASONRY STRUCTURES".
5. USE CONCRETE MASONRY UNITS CONFORMING TO ASTM C90. PROVIDE F'M OF 2000 PSI (UNIT STRENGTH 2000 PSI) FOR ALL REINFORCED MASONRY WALLS. PERFORM COMPRESSIVE STRENGTH COMPLIANCE BY PRISM TEST METHOD. USE ONLY MASONRY UNITS THAT ARE A MIN. OF 50% SOLID. REFER TO THE SPECIFICATIONS FOR TESTING FREQUENCIES.
6. USE TYPE "S" MORTAR IN ACCORDANCE WITH ASTM C270. USE FULL-BEDDED JOINTS FOR ALL MASONRY UNITS. REMOVE MORTAR PROTRUDING INTO CELL CAVITIES THAT ARE TO BE REINFORCED AND GROUTED. ALLOW A MIN. OF 24 HOURS FOR MORTAR TO CURE BEFORE PLACING GROUT. REFER TO THE SPECIFICATIONS FOR TESTING REQUIREMENTS.
7. USE ALL GROUT CONFIRMING TO ASTM C-476 WITH A MIN. COMPRESSIVE STRENGTH OF 3000 PSI IN 28 DAYS, TESTED IN ACCORDANCE WITH ASTM C1019. AGGREGATE TO CONFORM TO ASTM C404 FOR COARSE GROUT AND SLUMP OF 8" TO 11". TEST SAMPLES FOR COMPRESSIVE STRENGTH. REFER TO THE SPECIFICATION FOR TESTING REQUIREMENTS.
8. REFER TO THE MASONRY DETAILS FOR REINFORCING REQUIREMENTS.
9. FOR UNREINFORCED WALLS USE STANDARD TRUSS-TYPE MASONRY HORIZONTAL REINFORCING IN EVERY OTHER COURSE OF MASONRY.
10. USE ASTM A-615 GRADE 60 REINFORCING STEEL.
11. IN HIGH-LIFT GROUTING USE A MAX. LIFT OF 5'-4" WITH MIN. HALF HOUR MAX. ONE HOUR BETWEEN LIFTS. VIBRATE EACH LIFT AND RECONSOLIDATE PREVIOUS LIFT AFTER PLACING NEXT LIFT.
12. WHERE ANCHOR BOLTS ARE SET IN MASONRY WALL, FILL BLOCK CELLS WITH GROUT FOR BOLTED COURSE, ONE COURSE ABOVE AND TWO COURSES BELOW ANCHOR ELEVATION.
13. USE PRESSURE-TREATED WOOD FOR ALL WOOD IN CONTACT WITH MASONRY.
14. UNLESS OTHERWISE NOTED, PROVIDE LINTELS OR HEADERS OVER ALL MASONRY OPENINGS NOT FLUSH WITH STRUCTURAL FRAME. LINTELS OR HEADERS TO BEAR MINIMUM 16 INCHES EACH SIDE OF OPENING. REFER TO TYPICAL DETAILS.
15. FOR WALLS REQUIRING A FIRE RESISTANCE RATING, PROVIDE TO THE ARCHITECT, A CERTIFICATION INDICATING THAT THE MANUFACTURER OF THE CONCRETE MASONRY UNITS HAS COMPLIED WITH ALL THE REQUIREMENTS OF THE UL LISTINGS AS SPECIFIED ON THE ARCHITECTURAL DRAWINGS.
16. COORDINATE WITH THE ARCH DRAWINGS FOR MASONRY LAYOUT & LOCATIONS OF OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS.

STATEMENT OF SPECIAL INSPECTIONS

SPECIAL INSPECTIONS:

- 1. SPECIAL INSPECTIONS ARE REQUIRED IN ACCORDANCE WITH 2018 INTERNATIONAL BUILDING CODE, CHAPTER 17.
2. THE OWNER SHALL SELECT AND PAY ALL COSTS OF EMPLOYING A SPECIAL INSPECTOR, BUT THE SPECIAL INSPECTOR SHALL BE RESPONSIBLE TO THE ENFORCING AGENCY.
3. THE CONTRACTOR'S CONTRACTUAL OR STATUTORY OBLIGATIONS ARE NOT RELIEVED BY ANY ACTION OF THE SPECIAL INSPECTOR.
4. SPECIAL INSPECTION FOR WIND: REQUIRED
5. SPECIAL INSPECTIONS FOR SEISMIC: NOT REQUIRED

CONCRETE CONSTRUCTION:

- 1. PERIODIC SPECIAL INSPECTION SHALL BE PERFORMED ON THE FOLLOWING:
A. REINFORCING STEEL SIZE, SPACING AND PLACEMENT
B. ANCHOR BOLTS SIZE, SPACING, AND EMBEDMENT.
C. POST INSTALLED ANCHORS SIZE, SPACING, EMBEDMENT, AND PROPER INSTALLATION TECHNIQUES.
D. VERIFY CONCRETE TRUCK TICKETS ARE PROVIDING APPROVED MIX DESIGN.
2. CONTINUOUS SPECIAL INSPECTIONS SHALL BE PERFORMED ON THE FOLLOWING:
A. CONCRETE SLUMP, AIR CONTENT, AND TEMPERATURE, AND TEST CYLINDER ACCORDING TO CONTRACT DOCUMENTS AND APPROVED MIX DESIGN.
B. CONCRETE PLACEMENT FOR PROPER TECHNIQUE.

WOOD CONSTRUCTION:

- 1. SPECIAL INSPECTIONS OF PREFABRICATED WOOD STRUCTURAL ELEMENTS AND ASSEMBLIES SHALL BE IN ACCORDANCE WITH SECTION 1704.2.5.

PRE-CONSTRUCTION TESTS:

- 1. REFER TO THE TECHNICAL SPECIFICATIONS FOR REQUIRED MATERIAL AND ASSEMBLY TESTS FOR THIS PROJECT.

STRUCTURAL OBSERVATIONS:

- 1. THE OWNER SHALL EMPLOY A REGISTERED DESIGN PROFESSIONAL LICENSED IN THE STATE THE PROJECT IS LOCATED TO VISUALLY OBSERVE THE STRUCTURAL SYSTEM FOR GENERAL CONFORMANCE TO THE APPROVED CONSTRUCTION DOCUMENTS.
2. STRUCTURAL OBSERVATIONS DOES NOT REPLACE THE TESTING AND INSPECTION REQUIREMENTS OF THE CONTRACT DOCUMENTS.
3. PRIOR TO COMMENCEMENT OF OBSERVATIONS, THE STRUCTURAL OBSERVER SHALL SUBMIT TO THE BUILDING OFFICIAL A WRITTEN STATEMENT IDENTIFYING HIS/HER QUALIFICATIONS ALONG WITH IDENTIFYING THE FREQUENCY AND EXTENT OF STRUCTURAL OBSERVATIONS.
4. AT THE CONCLUSION OF THE WORK INCLUDED IN THE PERMIT, THE STRUCTURAL OBSERVER SHALL SUBMIT TO THE BUILDING OFFICIAL A WRITTEN STATEMENT THAT THE SITE VISITS HAVE BEEN MADE AND IDENTIFY AND REPORTED DEFICIENCIES WHICH, TO THE BEST OF THE STRUCTURAL OBSERVER'S KNOWLEDGE, HAVE NOT BEEN RESOLVED.

MISCELLANEOUS STEEL:

- 1. FOR MISCELLANEOUS STEEL NOT SHOWN ON STRUCTURAL DRAWINGS, REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS.
2. EDGE ANGLES, CLIP ANGLES, PLATES, BARS AND OTHER MISCELLANEOUS ROLLED SHAPES SHALL BE 316 STAINLESS STEEL, UNLESS OTHERWISE NOTED.

TEMPORARY BRACING:

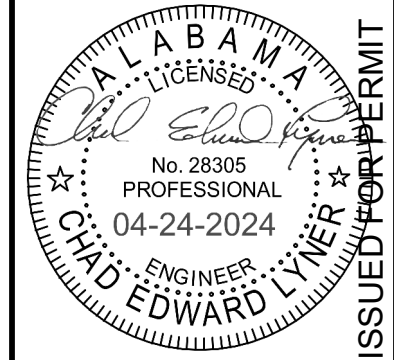
- 1. THE CONTRACTOR SHALL PROVIDE TEMPORARY BRACING AS REQUIRED DURING CONSTRUCTION.
2. THE CONTRACTOR SHALL RETAIN AT THE CONTRACTOR'S EXPENSE A REGISTERED STRUCTURAL ENGINEER LICENSED IN THE STATE OF THE PROJECT TO DESIGN AND INSPECT ALL TEMPORARY SHORING AND BRACING. SIGNED, SEALED AND DATED DESIGN CALCULATIONS SHALL BE SUBMITTED FOR REVIEW WHEN REQUESTED.

STANDARD STRUCTURAL ABBREVIATIONS

Table with 4 columns: Abbreviation, Full Name, Abbreviation, Full Name. Includes entries like AB ANCHOR BOLT, EXP ANCHOR BOLT, ADD ADDITIVE, etc.

MOTT MACDONALD logo and contact information: 107 St. Francis Street, Suite 2900, Mobile, Alabama 36602. Telephone: (251) 343-4398, Fax: (251) 343-6902. Architects, Engineers, Surveyors.

CHRISTIANPREUS Landscape Architecture logo. ARCHITECTURAL DRAWINGS FOR: CITY OF MOBILE - MIMS PARK CONCESSIONS. Mobile, AL 36693. SCALE 12" = 1'-0". DATE: May 5, 2024. ISSUED FOR PERMIT. S0.2



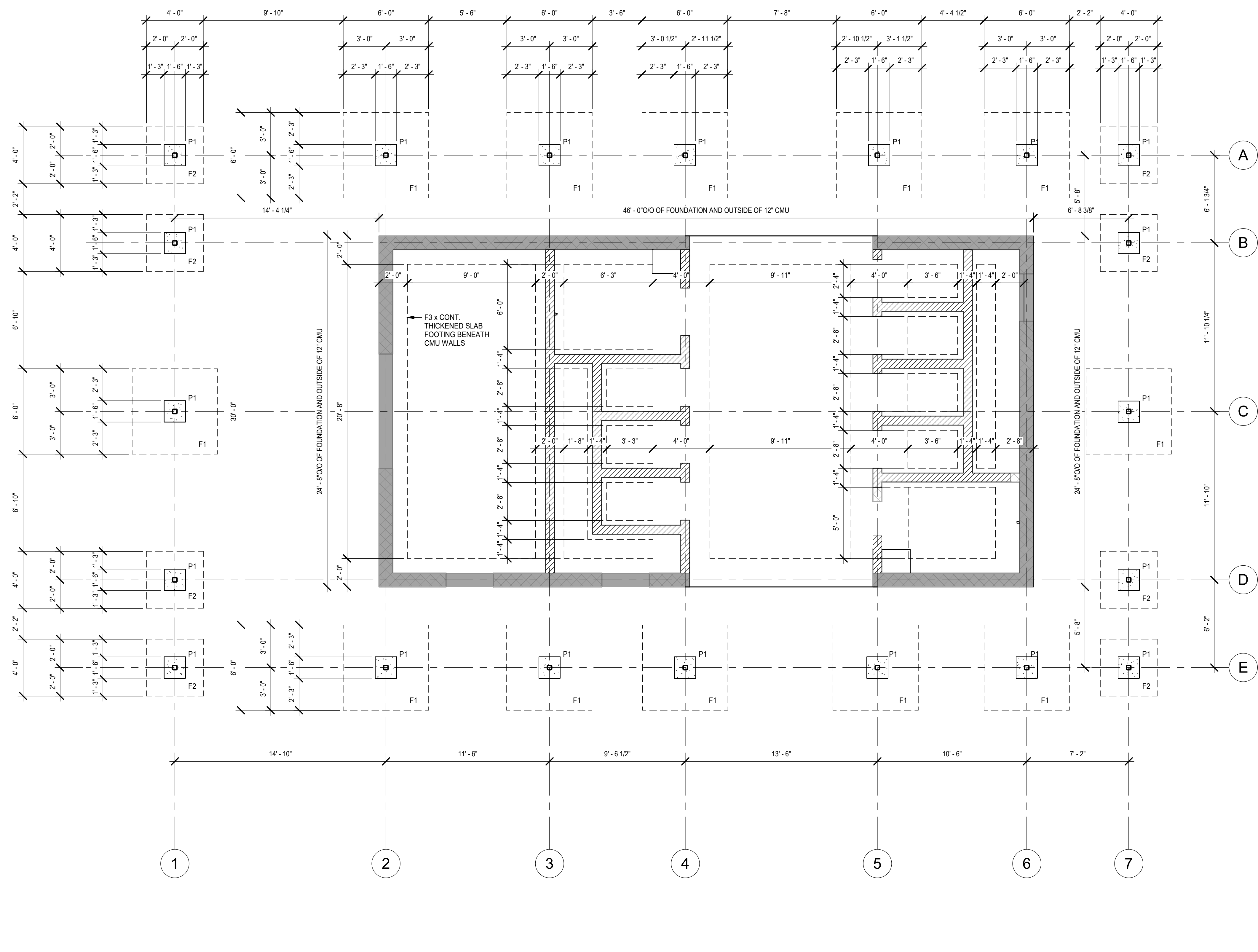
**M M**  
**MOTT**  
**MACDONALD**

107 St. Francis Street  
 Suite 2000,  
 Mobile, Alabama 36602  
 Telephone: (251) 343-4398  
 Fax: (251) 343-6902  
 Architects  
 Engineers  
 Surveyors

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ARCHITECTURAL DRAWINGS FOR:  
**CITY OF MOBILE - MIMS PARK CONCESSIONS**  
 Mobile, AL 36693

DATE: May 5, 2024  
 SCALE: As indicated  
 ISSUED FOR PERMIT  
**S1.1**



### FOOTING SCHEDULE

MARK	SIZE SHORT x LONG x THICKNESS	TOP		BOTTOM		REMARKS
		SHORT	LONG	SHORT	LONG	
F1	6'-0" x 6'-0" x 1'-6"	(7)#6	(7)#6	(7)#6	(7)#6	
F2	4'-0" x 4'-0" x 1'-6"	(5)#6	(5)#6	(5)#6	(5)#6	
F3	STRIP FTG-CONTx 2'-0" DEEP x WIDTH VARIES	#4@12" O.C.	(4)#4	#4@12" O.C.	(4)#4	

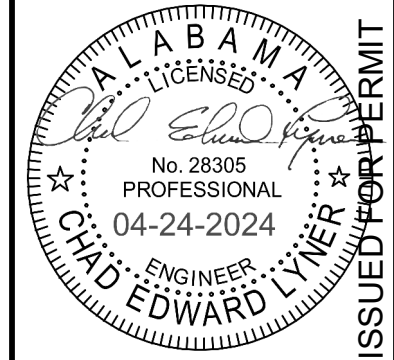
### PIER SCHEDULE

MARK	SIZE SHORT x LONG x THICKNESS	LONG	TIES T&B	INTERMEDIATE
			#4 TIES	TIES
P1	1'-6"x1'-6"x1'-6"	(10)#7 EQ. SPACED AROUND PERIMETER	3 TIES @ 2' O.C. @ T&B	3 TIES EQUALLY SPACED

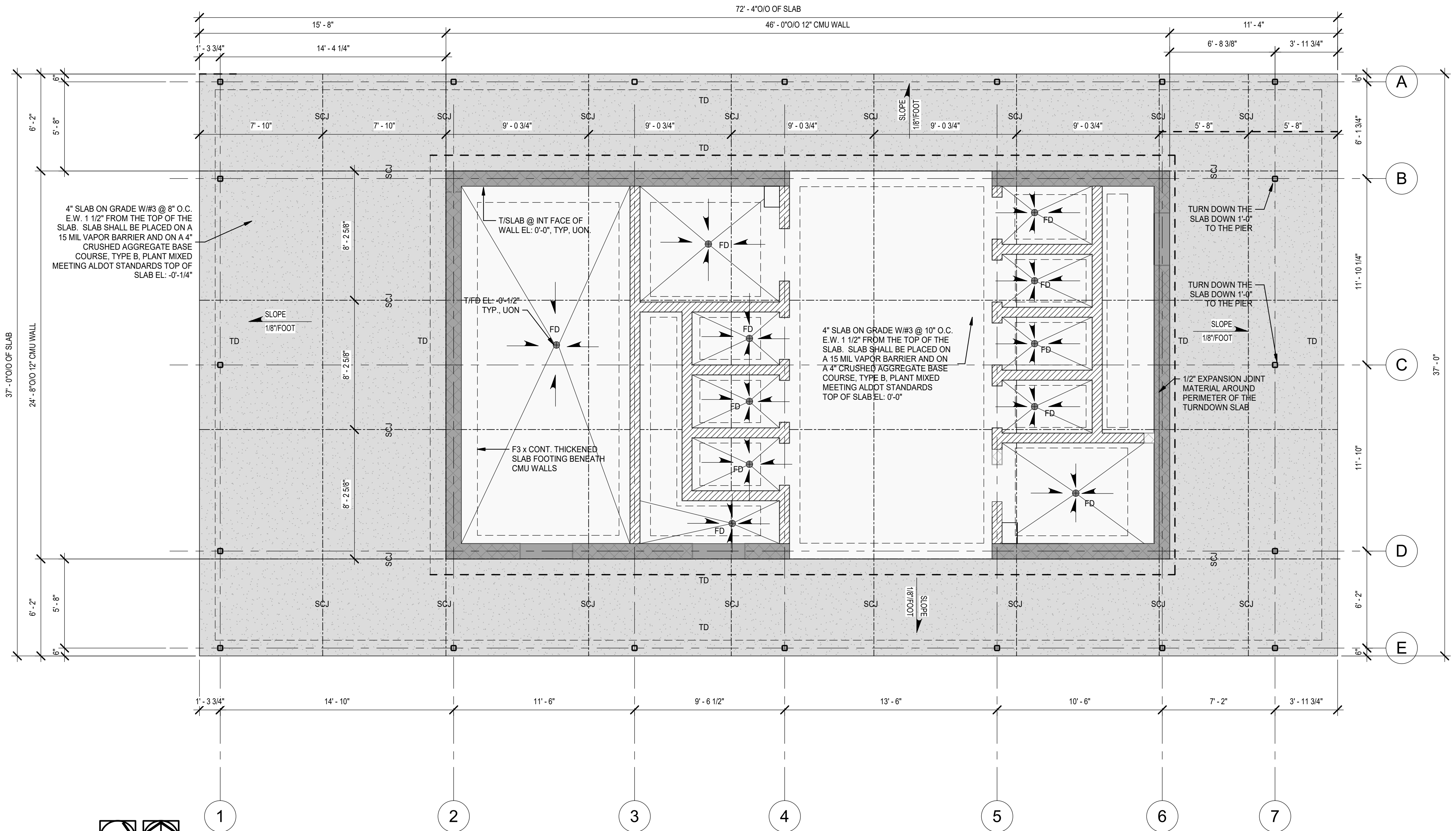
- FOUNDATION NOTES:**
- REFER TO GENERAL NOTES FOR NGVD REFERENCE ELEVATION FOR STRUCTURAL PLANS.
  - TOP OF SLAB ELEVATION: 0'-0" UON
  - TOP OF FOUNDATION ELEVATION: -3'-0" UON.
  - TOP OF PIER ELEVATION: -1'-0" UON.
  - ALL COLUMNS AND PIERS ARE CENTERED ON THE THE COLUMN GRID UON.
  - ALL COLUMN AND PIERS ARE CENTERED ON THE FOUNDATIONS UON.
  - ALL WALLS SHALL BE CENTERED ON THE STRIP FOOTINGS UON.
  - START AND END REINF WITH CLEAR COVER NOT TO EXCEED MIN. ALLOWED COVER ON ALL SIDES OF THE FOOTING. REMAINDER OF REINF SHALL BE PLACED WITH NO BAR SPACING EXCEEDING SPACING SHOWN IN THE SCHEDULE.
  - LONG REINF REFERS TO THE LONGER LENGTH BARS PLACED ACROSS THE SHORT SIDE.
  - SHORT REINF REFERS TO THE SHORTER LENGTH BARS PLACED ACROSS THE LONG SIDE.
  - FOOTING SIZE SHOWN IS A MAX OUTSIDE DIMENSIONS AND THICKNESS. REFER TO PLAN FOR ACTUAL SHAPE AND ORIENTATION.

- LEGEND**
- DENOTES CMU WALL - 8" UON (T/WALL CMU WALL EL: 10'-0" (TYP))
  - DENOTES CMU WALL - 12" UON (T/WALL CMU WALL VARIES BETWEEN 14'-10" LOW END AND 20'-0" HIGH END(TYP))
  - INDICATES FOUNDATION TYPE  
REFER TO FOOTING SCHEDULE
  - INDICATES PIER TYPE  
REFER TO PIER SCHEDULE
  - INDICATES TOP OF FOOTING REFERENCE ELEVATION  
REFER TO GENERAL NOTES
  - FLOOR DRAIN  
TOP OF FLOOR DRAIN ELEVATION: -0'-1/2"
  - FLOOR SLOPE TOWARD THE FLOOR DRAIN
  - TURNDOWN, REFER TO SHEET S3.1 FOR TURNDOWN DETAILS
  - SAW CUT JOINT, REFER TO SHEET S3.1 FOR SAW CUT DETAILS
  - HSS 4x4x3/8" COLUMNS

**FOUNDATION PLAN**  
 SCALE: 1/4" = 1'-0"  
 TRUE NORTH PROJECT NORTH







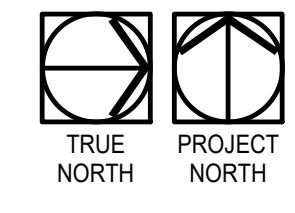
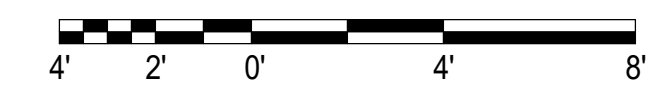
**SLAB ON GRADE NOTES:**

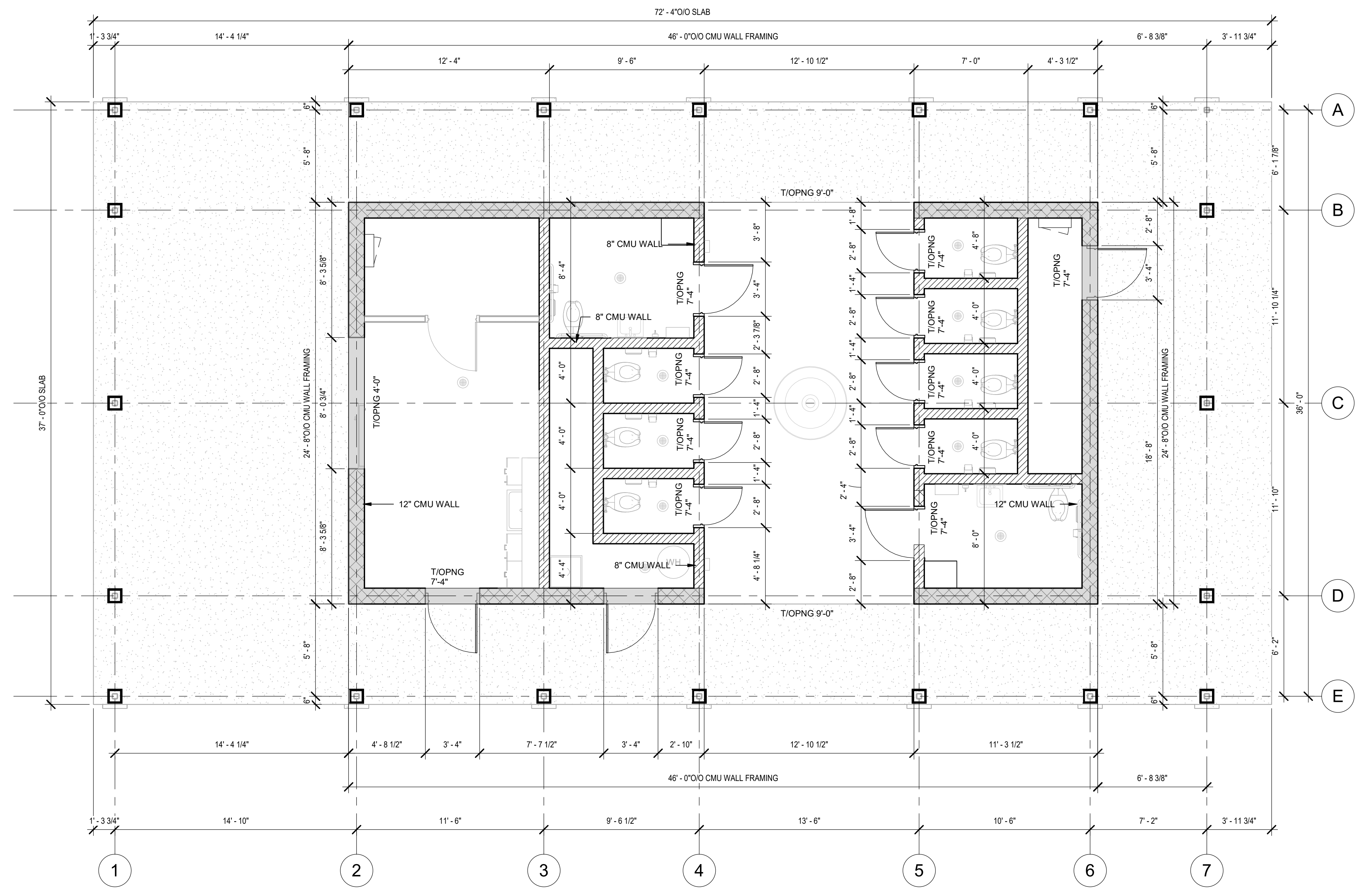
1. REFER TO GENERAL NOTES SHEET FOR ADDITIONAL CONCRETE AND SLAB ON GRADE NOTES.
2. SLAB ON GRADE SHALL BE MINIMUM 4" THICK CONCRETE PLACED ON 15 MIL MINIMUM VAPOR BARRIER ON COMPACTED FILL. REINFORCE SLAB ON GRADE WITH #3 @ 8" ON CENTER EACH WAY. REFER TO TYPICAL DETAILS FOR ADDITIONAL REINFORCEMENT AND REQUIREMENTS. USE CHAIRS TO POSITION REINFORCING 1" BELOW TOP OF SLAB, UNLESS OTHERWISE INDICATED, AND TO MAINTAIN THAT DEPTH DURING CONCRETE PLACEMENT.
3. TOP OF SLAB ELEVATION = 0'-0" (97.00'), UNLESS OTHERWISE NOTED. REFER TO GENERAL NOTES FOR NGVD REFERENCE ELEVATION.
4. COORDINATE ALL SLAB PENETRATION SIZES AND LOCATIONS WITH ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS.
5. CONTRACTOR SHALL SUBMIT CONTROL / CONSTRUCTION JOINT LOCATION PLAN TO EOR FOR REVIEW PRIOR TO PLACING. REFER TO S3.1.
6. REFER TO S3.1 FOR TYPICAL SLAB ON GRADE DETAILS.
7. "-" INDICATES THICKNESS OF CONC ABOVE T/SLAB.
8. COORDINATE RECESSED SLAB LOCATIONS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
9. SLOPE TOP OF SLAB TO FLOOR DRAINS.

**LEGEND**

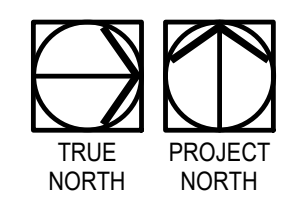
- EXTERIOR 4" SLAB (TOP OF SLAB EL: -0'-1/4")
- INTERIOR 4" SLAB AND STRIP FOUNDATION SYSTEM (TOP OF SLAB EL: 0'-0")
- DENOTES CMU WALL - 8" UON (T/WALL CMU WALL EL: 10'-0" (TYP))
- DENOTES CMU WALL - 12" UON (T/WALL CMU WALL EL: 10'-0" (TYP))
- INDICATES FOUNDATION TYPE REFER TO FOOTING SCHEDULE
- INDICATES PIER TYPE REFER TO PIER SCHEDULE
- INDICATES TOP OF FOOTING REFERENCE ELEVATION REFER TO GENERAL NOTES
- FLOOR DRAIN TOP OF FLOOR DRAIN ELEVATION: -0'-1/2"
- FLOOR SLOPE TOWARD THE FLOOR DRAIN
- TURNDOWN, REFER TO SHEET S3.1 FOR TURNDOWN DETAILS
- SAW CUT JOINT, REFER TO SHEET S3.1 FOR SAW CUT DETAILS
- HSS 4x4x3/8" COLUMNS

**SLAB ON GRADE PLAN**  
SCALE: 1/4" = 1'-0"





**CMU WALL PLAN**  
SCALE: 1/4" = 1'-0"



**SLAB ON GRADE NOTES:**

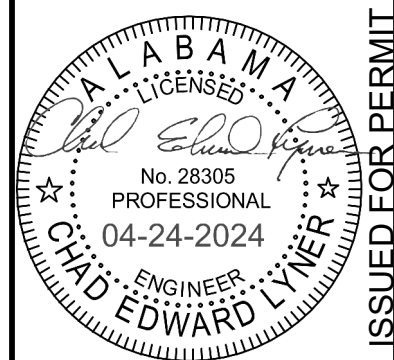
1. REFER TO GENERAL NOTES SHEET FOR ADDITIONAL CONCRETE AND SLAB ON GRADE NOTES.
2. SLAB ON GRADE SHALL BE MINIMUM 4" THICK CONCRETE PLACED ON 15 MIL MINIMUM VAPOR BARRIER ON COMPACTED FILL. REINFORCE SLAB ON GRADE WITH #3 @ 8" ON CENTER EACH WAY. REFER TO TYPICAL DETAILS FOR ADDITIONAL REINFORCEMENT AND REQUIREMENTS. USE CHAIRS TO POSITION REINFORCING 1" BELOW TOP OF SLAB, UNLESS OTHERWISE INDICATED, AND TO MAINTAIN THAT DEPTH DURING CONCRETE PLACEMENT.
3. TOP OF SLAB ELEVATION = 0'-0" (147.50'), UNLESS OTHERWISE NOTED. REFER TO GENERAL NOTES FOR NGVD REFERENCE ELEVATION.
4. COORDINATE ALL SLAB PENETRATION SIZES AND LOCATIONS WITH ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS.
5. CONTRACTOR SHALL SUBMIT CONTROL / CONSTRUCTION JOINT LOCATION PLAN TO EOR FOR REVIEW PRIOR TO PLACING. REFER TO S3.1.
6. REFER TO S3.2 FOR TYPICAL SLAB ON GRADE DETAILS.
7. "+-" INDICATES THICKNESS OF CONC ABOVE T/SLAB.
8. COORDINATE RECESSED SLAB LOCATIONS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
9. SLOPE TOP OF SLAB TO FLOOR DRAINS.

**TYPICAL CMU WALL NOTES:**

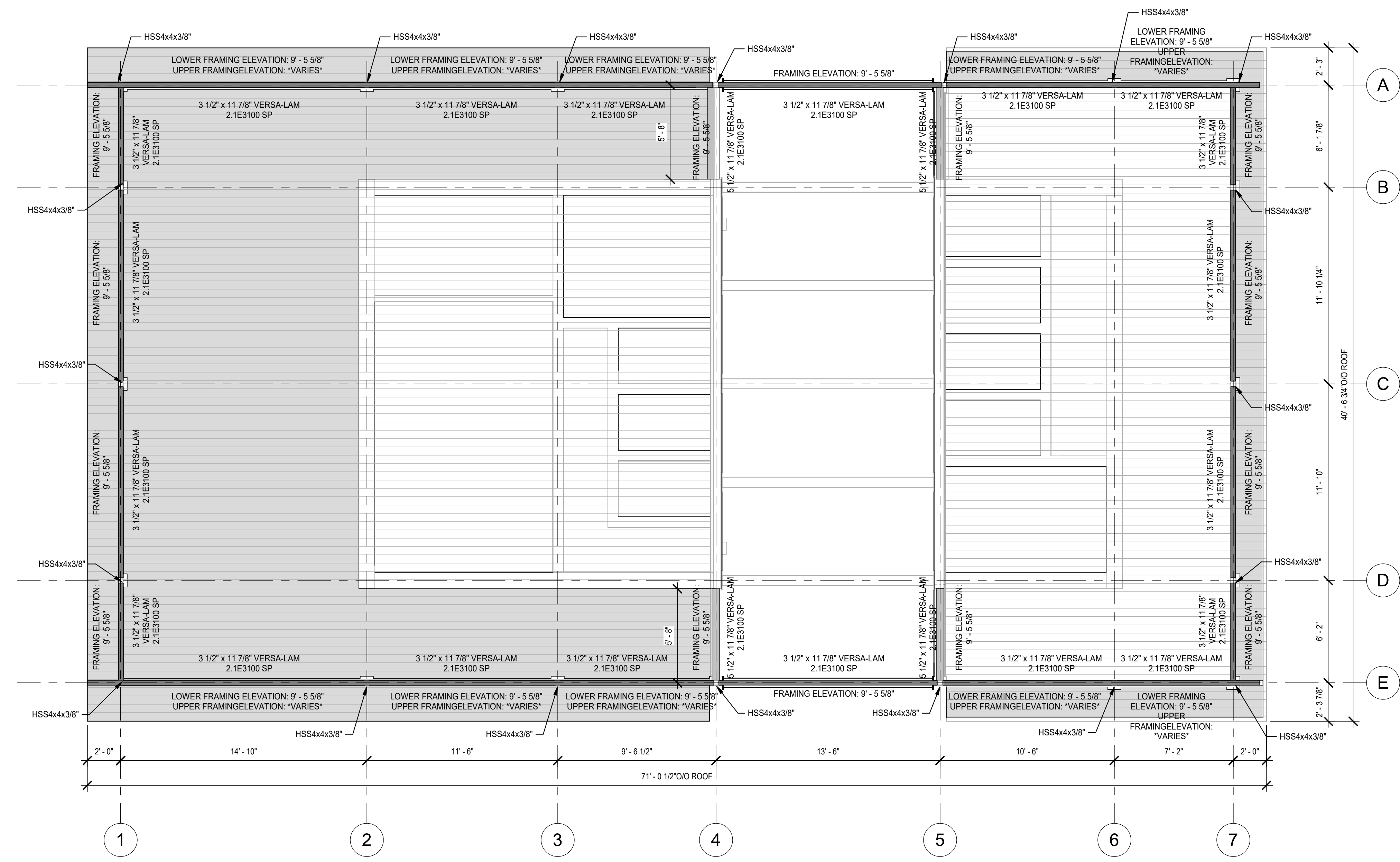
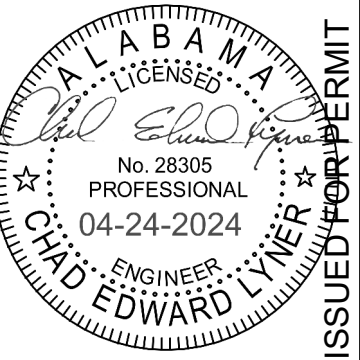
1. TOP OF CMU WALL ELEVATION = VARIES, UNLESS OTHERWISE NOTED. REFER TO GENERAL NOTES FOR NGVD REFERENCE ELEVATION.
2. REFER TO GENERAL NOTES AND STANDARD CMU SHEETS FOR ADDITIONAL CMU INFORMATION.
3. REFER TO SHEET SXXX FOR STANDARD CMU DETAILS AND REINFORCING
4. REFER TO ARCHITECTURAL, ELECTRICAL, MECHANICAL AND PLUMBING PLANS FOR OPENINGS IN CMU WALLS.
5. COORDINATE ALL OPENINGS AND OPENING ELEVATIONS IN THE CMU WITH THE ARCHITECTURAL, ELECTRICAL, MECHANICAL AND PLUMBING DRAWINGS.
6. PROVIDE MINIMUM 16" BOND BEAM WITH (2) #5 TOP AND BOTTOM OVER ALL WALL OPENINGS UNLESS OTHERWISE DESIGNATED.

**LEGEND**

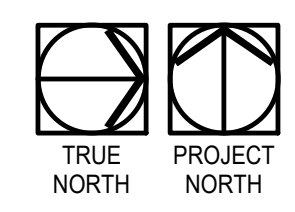
- DENOTES 12" CMU WALL
- DENOTES 8" CMU WALL
- INDICATES TURNDOWN SLAB  
REFER TO SHEET S7.7 FOR DETAILS
- INDICATES THICKENED SLAB  
REFER TO SHEET S3.2 FOR DETAILS
- INDICATES TOP OF SLAB REFERENCE ELEVATION  
REFER TO GENERAL NOTES
- INDICATES HSS4X4X3/8 COLUMN  
ATTACH BICOL TO SUPPORT w/ SIMPSON MPB88Z MOMENT POST BASE
- INDICATES SLAB STEP DOWN
- INDICATES SLAB SLOPE DOWN
- INDICATES (43) 3/4" DIA. GALVANIZED THREADED RODS WITH GALV. NUT AND WASHER EACH END AND A COUPLING CONNECTION AT 2'-0" ABOVE THE FINISHED FLOOR AROUND THE PERIMETER OF THE BUILDING. ANCHOR ONE NUT AND WASHER A MINIMUM OF 10" INTO THE CONCRETE TURNDOWN FOUNDATION.







**PERIMETER GIRDER FRAMING PLAN**  
SCALE: 1/4" = 1'-0"



**SLAB ON GRADE NOTES:**

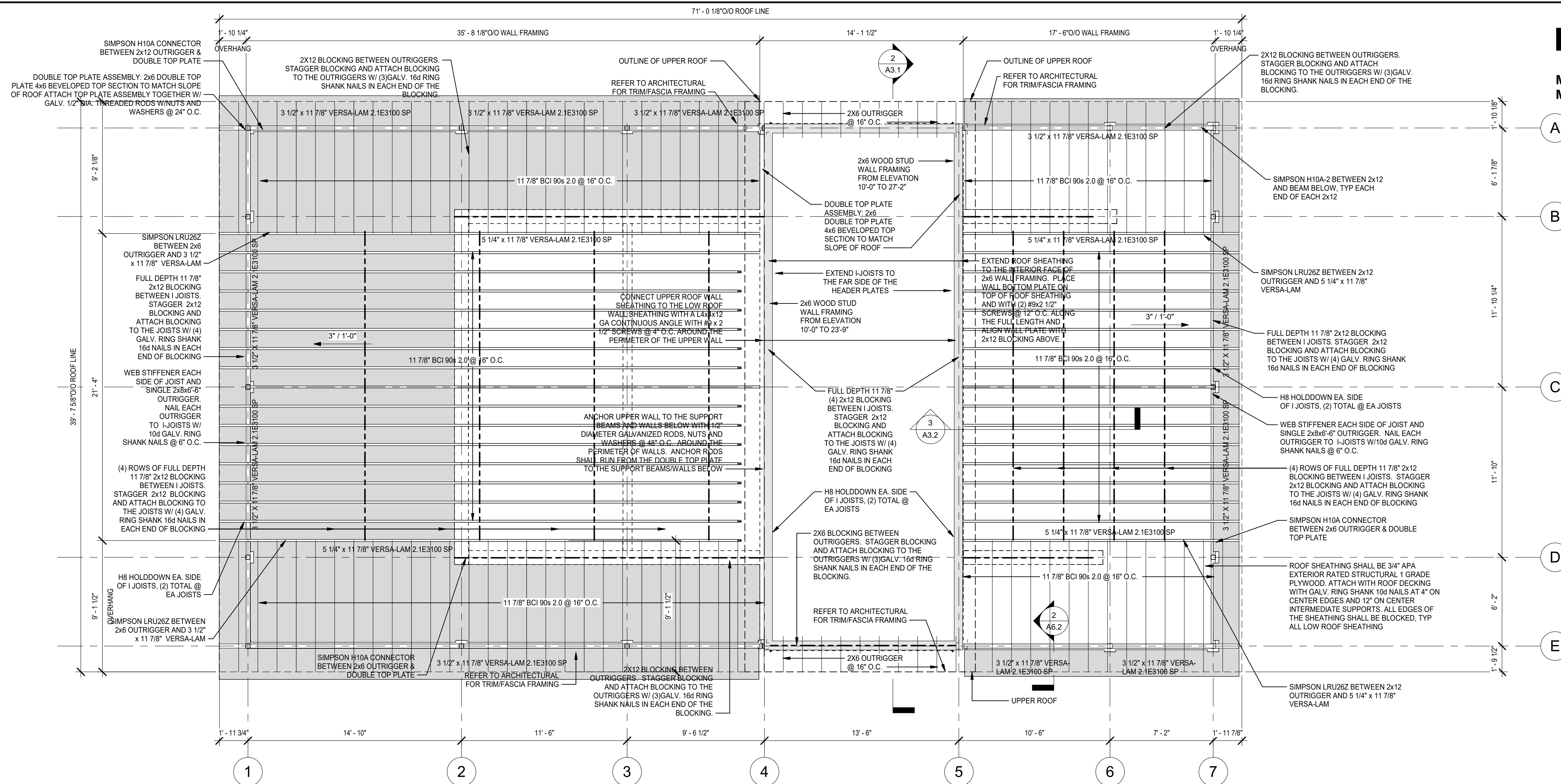
1. REFER TO GENERAL NOTES SHEET FOR ADDITIONAL CONCRETE AND SLAB ON GRADE NOTES.
2. SLAB ON GRADE SHALL BE MINIMUM 4" THICK CONCRETE PLACED ON 15 MIL MINIMUM VAPOR BARRIER ON COMPACTED FILL. REINFORCE SLAB ON GRADE WITH #3 @ 8" ON CENTER EACH WAY. REFER TO TYPICAL DETAILS FOR ADDITIONAL REINFORCEMENT AND REQUIREMENTS. USE CHAIRS TO POSITION REINFORCING 1" BELOW TOP OF SLAB, UNLESS OTHERWISE INDICATED, AND TO MAINTAIN THAT DEPTH DURING CONCRETE PLACEMENT.
3. TOP OF SLAB ELEVATION = 0'-0" (147.50'), UNLESS OTHERWISE NOTED. REFER TO GENERAL NOTES FOR NGVD REFERENCE ELEVATION.
4. COORDINATE ALL SLAB PENETRATION SIZES AND LOCATIONS WITH ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS.
5. CONTRACTOR SHALL SUBMIT CONTROL / CONSTRUCTION JOINT LOCATION PLAN TO EOR FOR REVIEW PRIOR TO PLACING. REFER TO S3.1.
6. REFER TO S3.2 FOR TYPICAL SLAB ON GRADE DETAILS.
7. "+" INDICATES THICKNESS OF CONC ABOVE T/SLAB.
8. COORDINATE RECESSED SLAB LOCATIONS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
9. SLOPE TOP OF SLAB TO FLOOR DRAINS.

**TYPICAL CMU WALL NOTES:**

1. TOP OF CMU WALL ELEVATION = VARIES, UNLESS OTHERWISE NOTED. REFER TO GENERAL NOTES FOR NGVD REFERENCE ELEVATION.
2. REFER TO GENERAL NOTES AND STANDARD CMU SHEETS FOR ADDITIONAL CMU INFORMATION.
3. REFER TO SHEET SXXXX FOR STANDARD CMU DETAILS AND REINFORCING
4. REFER TO ARCHITECTURAL, ELECTRICAL, MECHANICAL AND PLUMBING PLANS FOR OPENINGS IN CMU WALLS.
5. COORDINATE ALL OPENINGS AND OPENING ELEVATIONS IN THE CMU WITH THE ARCHITECTURAL, ELECTRICAL, MECHANICAL AND PLUMBING DRAWINGS.
6. PROVIDE MINIMUM 16" BOND BEAM WITH (2) #5 TOP AND BOTTOM OVER ALL WALL OPENINGS UNLESS OTHERWISE DESIGNATED.

**LEGEND**

- DENOTES 3 1/2" x 11 7/8" VERSA-LAM 2.1E3100 SP
- DENOTES 5 1/2" x 11 7/8" VERSA-LAM 2.1E3100 SP
- INDICATES TURNDOWN SLAB REFER TO SHEET S?/? FOR DETAILS
- INDICATES THICKENED SLAB REFER TO SHEET S3.2 FOR DETAILS
- INDICATES TOP OF SLAB REFERENCE ELEVATION REFER TO GENERAL NOTES
- INDICATES HSS4x4x3/8 COLUMN ATTACH B/COL TO SUPPORT w/ SIMPSON MPB88Z MOMENT POST BASE
- INDICATES SLAB STEP DOWN
- INDICATES SLAB SLOPE DOWN
- INDICATES (43) 3/4" DIA. GALVANIZED THREADED RODS WITH GALV. NUT AND WASHER EACH END AND A COUPLING CONNECTION AT 2'-0" ABOVE THE FINISHED FLOOR AROUND THE PERIMETER OF THE BUILDING. ANCHOR ONE NUT AND WASHER A MINIMUM OF 10" INTO THE CONCRETE TURNDOWN FOUNDATION.



**M M**  
**MOTT**  
**MACDONALD**

107 St. Francis Street  
Suite 2900,  
Mobile, Alabama 36602  
Telephone: (251) 343-4398  
Fax: (251) 343-6902  
Architects  
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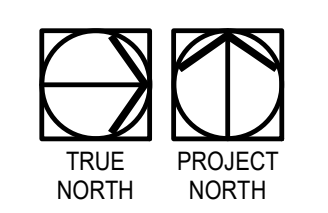
ALABAMA  
No. 28305  
PROFESSIONAL  
04-24-2024  
ENGINEER  
CHD EDWARD LYNN

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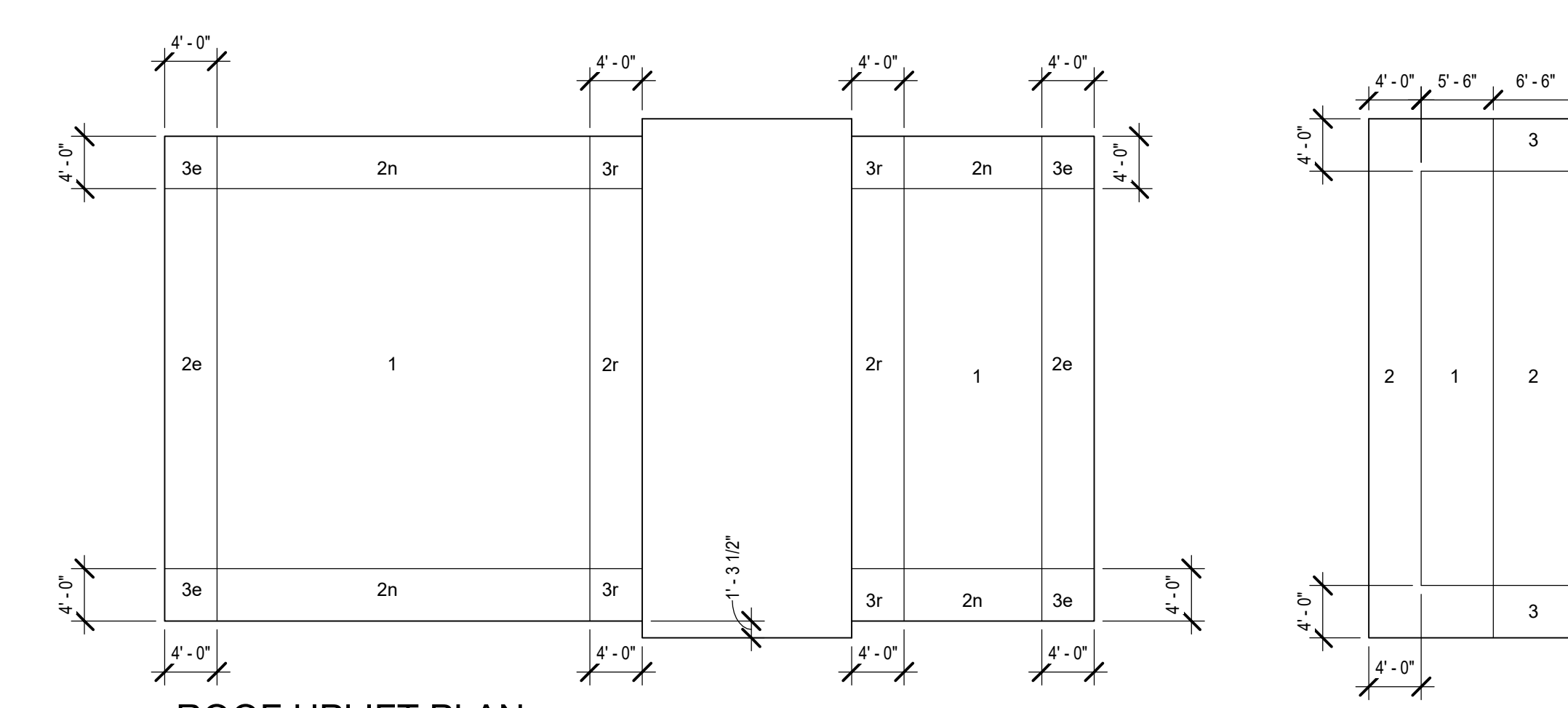
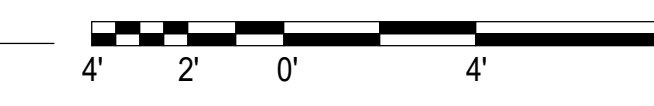
SCALE As indicated

DATE: May 5, 2024

**S1.5**



**ROOF FRAMING PLAN**  
SCALE: 1/4" = 1'-0"



**ROOF UPLIFT PLAN**  
SCALE: 3/32" = 1'-0"

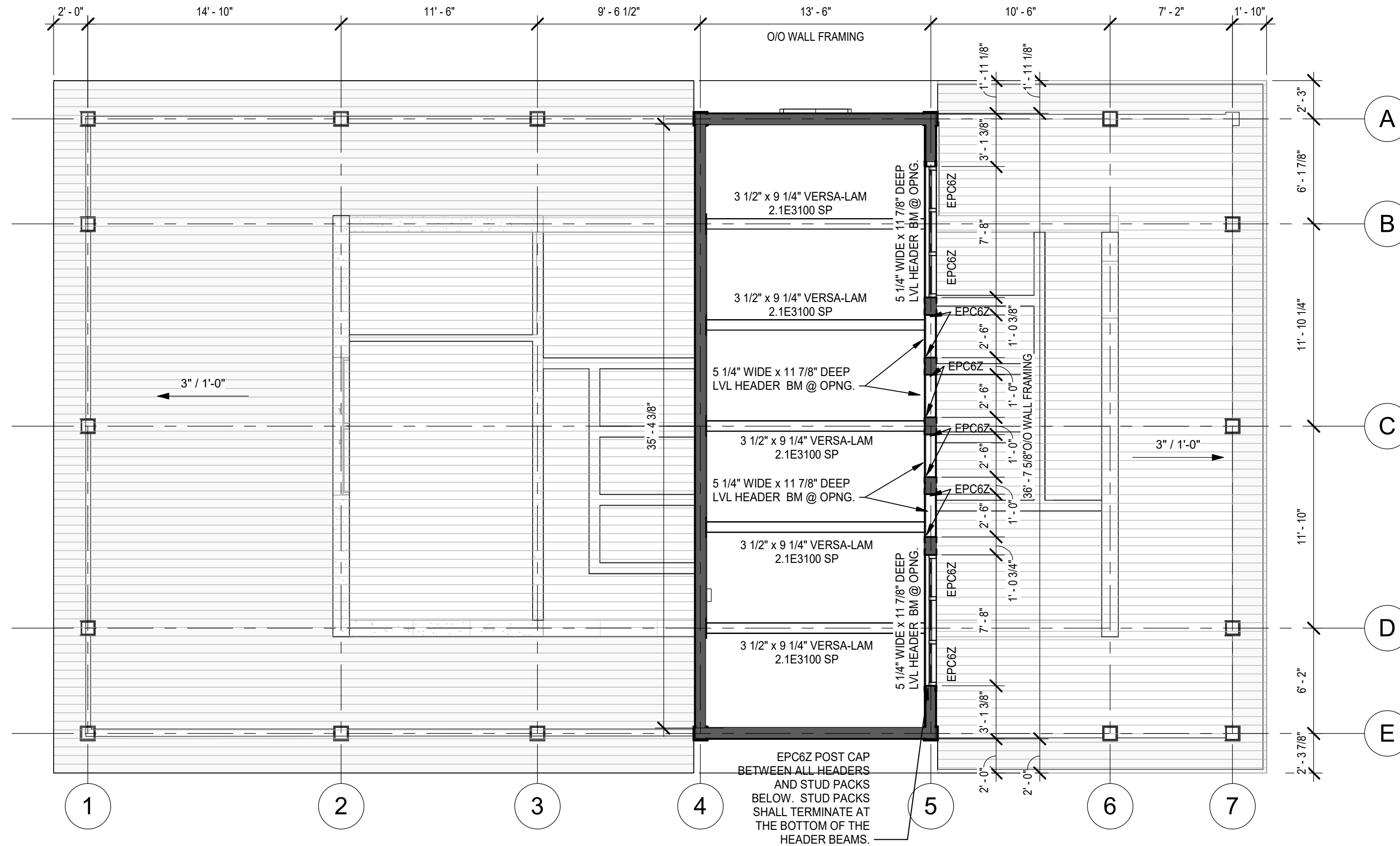
LOW GABLED ROOF PRESSURES				
ZONE	EFFECTIVE AREA (FT <sup>2</sup> )	PRESSURE (PSF)		
		POSITIVE	NEGATIVE	ROOF OVERHANG
1	<=10	33.5	-101.9	N/A
	50	25.8	-62.0	N/A
	100	22.4	-31.8	N/A
2e	<=10	33.5	-101.9	-125.3
	50	25.8	-62.0	-98.7
	100	22.4	-31.8	-78.6
2r	<=10	33.5	-148.7	-172.1
	50	25.8	-101.9	-137.0
	100	22.4	-81.8	-121.9
3e	<=10	33.5	-148.7	-200.1
	50	25.8	-101.9	-139.3
	100	22.4	-81.8	-113.2
3r	<=10	33.5	-176.8	-228.2
	50	25.8	-117.9	-149.7
	100	22.4	-92.6	-116.0

LOWER ROOF WALL PRESSURES			
ZONE	EFFECTIVE AREA (FT <sup>2</sup> )	PRESSURE (PSF)	
		POSITIVE	NEGATIVE
4	<=10	55.2	-59.9
	50	49.4	-54.1
	100	44.4	-49.1
5	<=10	55.2	-73.9
	50	49.4	-62.3
	100	44.4	-52.4

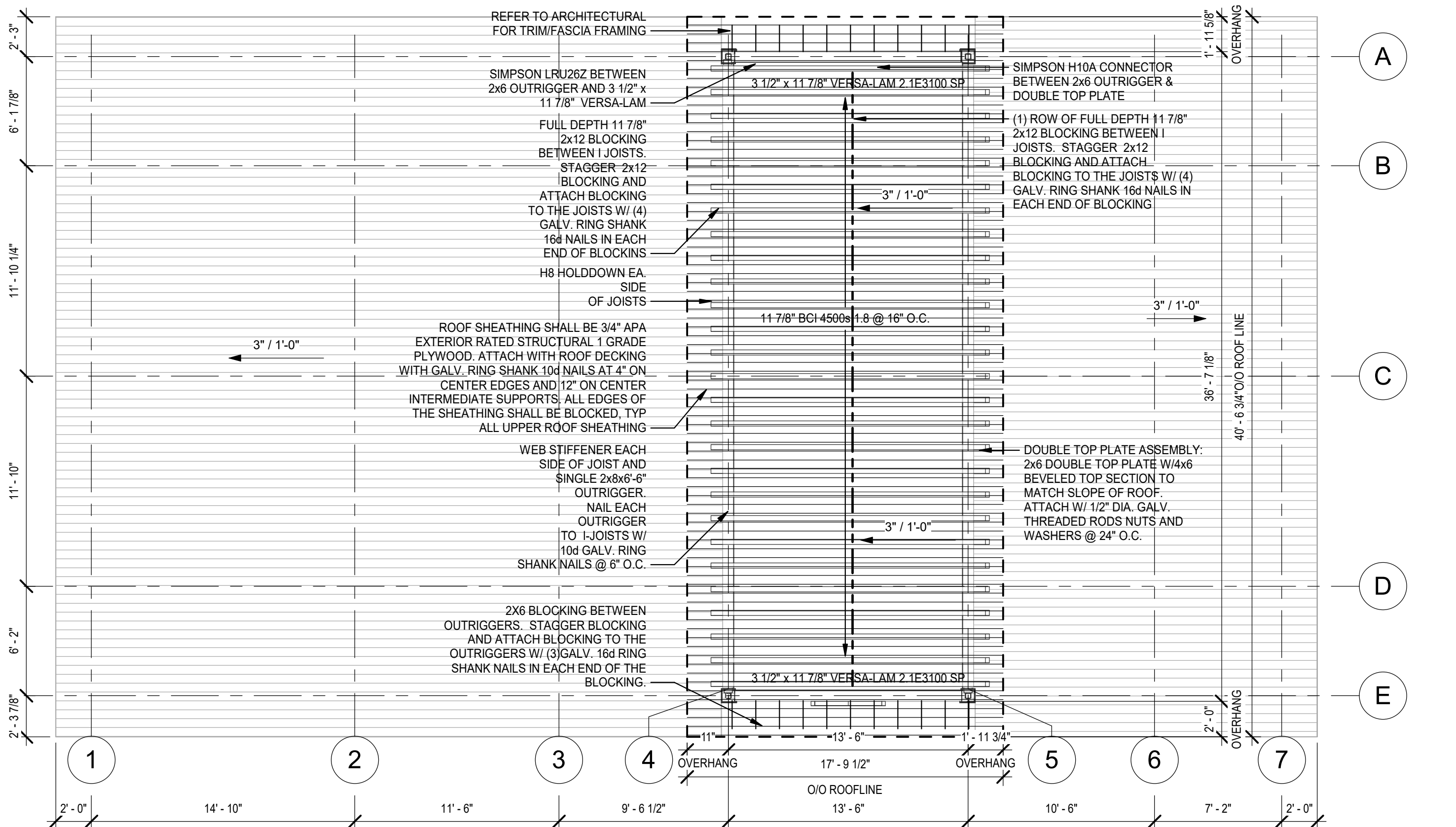
- NOTES:
- WALL SECTION 5 EXTENDS FROM THE BUILDING CORNERS A DISTANCE OF 4'-0". WALL SECTION 4 IS THE REMAINDER OF THE WALL.
  - COMPONENT AND CLADDING PRESSURES SHOWN ARE ULTIMATE PRESSURES AND CAN BE REDUCED BY 0.6 FOR ALLOWABLE PRESSURES.

- ROOF FRAMING NOTES:**
- ROOF SHEATHING SHALL BE 3/4" APA EXTERIOR RATED STRUCTURAL 1 GRADE PLYWOOD. ATTACH WITH ROOF DECKING WITH GALV. RING SHANK 10d NAILS AT 6" ON CENTER EDGES AND 12" ON CENTER INTERMEDIATE SUPPORTS. ALL EDGES OF THE SHEATHING SHALL BE BLOCKED. PROVIDE STAGGERED LAYOUT.
  - ALL WOOD MEMBERS EXPOSED TO WEATHER SHALL BE PRESSURE TREATED.
  - JOISTS BRACING SHOWN AS MINIMUM. REFER TO TRUSS DESIGNER FOR ADDITIONAL BRACING REQUIREMENTS.
  - ALL SIMPSON CONNECTORS AND HANGERS SHALL BE HOT DIPPED GALVANIZED WITH A MINIMUM OF A G90 COATING THICKNESS.
  - REFER TO ARCHITECTURAL DRAWINGS FOR TOP AND BOTTOM ELEVATIONS OF OPENINGS IN WALLS.
  - REFER TO S1.4 AND S1.6 FOR TOP OF WALL ELEVATIONS.
  - REFER TO S1.5 AND S1.6 FOR ROOF UPLIFT DIAGRAM AND PRESSURES.





UPPER WALL FRAMING PLAN  
SCALE: 3/16" = 1'-0"



UPPER ROOF FRAMING PLAN  
SCALE: 3/16" = 1'-0"

**M M**  
**MOTT**  
**MACDONALD**

107 St. Francis Street  
Suite 2900,  
Mobile, Alabama 36602  
Telephone: (251) 343-4398  
Fax: (251) 343-6902

Architects  
Engineers  
Surveyors

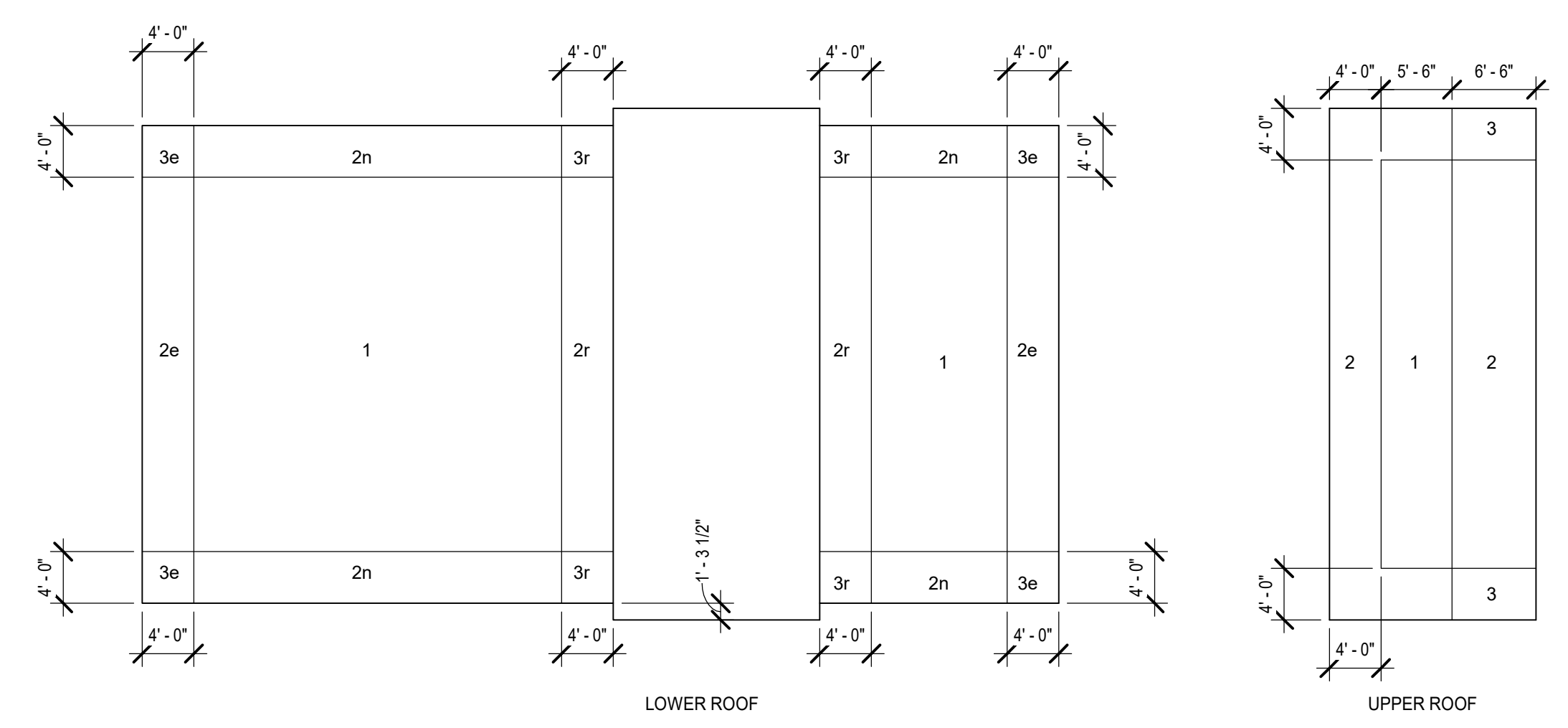
- ROOF FRAMING NOTES:**
1. ROOF SHEATHING SHALL BE 3/4" APA EXTERIOR RATED STRUCTURAL 1 GRADE PLYWOOD. ATTACH WITH ROOF DECKING WITH GALV. RING SHANK 10d NAILS AT 4" ON CENTER EDGES AND 12" ON CENTER INTERMEDIATE SUPPORTS. ALL EDGES OF THE SHEATHING SHALL BE BLOCKED. PROVIDE STAGGERED LAYOUT.
  2. ALL WOOD MEMBERS EXPOSED TO WEATHER SHALL BE PRESSURE TREATED.
  3. JOISTS BRACING SHOWN AS MINIMUM. REFER TO TRUSS DESIGNER FOR ADDITIONAL BRACING REQUIREMENTS.
  4. ALL SIMPSON CONNECTORS AND HANGERS SHALL BE HOT DIPPED GALVANIZED WITH A MINIMUM OF A G90 COATING THICKNESS.
  5. REFER TO ARCHITECTURAL DRAWINGS FOR TOP AND BOTTOM ELEVATIONS OF OPENINGS IN WALLS.
  6. REFER TO S1.4 AND S1.6 FOR TOP OF WALL ELEVATIONS.
  7. REFER TO S1.5 AND S1.6 FOR ROOF UPLIFT DIAGRAM AND PRESSURES.

LOW GABLED ROOF PRESSURES				
ZONE	EFFECTIVE AREA (FT <sup>2</sup> )	PRESSURE (PSF)		
		POSITIVE	NEGATIVE	ROOF OVERHANG
1	<=10	33.5	-101.9	N/A
	50	25.8	-62.0	N/A
	100	22.4	-31.8	N/A
	>200	22.4	-31.8	N/A
2e	<=10	33.5	-101.9	-125.3
	50	25.8	-62.0	-98.7
	100	22.4	-31.8	-78.6
	>200	22.4	-31.8	-78.6
2n	<=10	33.5	-148.7	-172.1
	50	25.8	-101.9	-137.0
	100	22.4	-81.8	-121.9
	>200	22.4	-61.7	-106.8
2r	<=10	33.5	-148.7	-172.1
	50	25.8	-101.9	-137.0
	100	22.4	-81.8	-121.9
	>200	22.4	-61.7	-106.8
3e	<=10	33.5	-148.7	-200.1
	50	25.8	-101.9	-139.3
	100	22.4	-81.8	-113.2
	>200	22.4	-61.7	-87.0
3r	<=10	33.5	-176.8	-228.2
	50	25.8	-117.9	-149.7
	100	22.4	-92.6	-116.0
	>200	22.4	-92.6	-116.0

LOWER ROOF WALL PRESSURES				
ZONE	EFFECTIVE AREA (FT <sup>2</sup> )	PRESSURE (PSF)		ROOF OVERHANG
		POSITIVE	NEGATIVE	
4	<=10	55.2	-59.9	
	50	49.4	-54.1	
	100	44.4	-49.1	
	>500	41.1	-45.8	
5	<=10	55.2	-73.9	
	50	49.4	-62.3	
	100	44.4	-52.4	
	>500	41.1	-45.8	

- NOTES:
1. WALL SECTION 5 EXTENDS FROM THE BUILDING CORNERS A DISTANCE OF 4'-0". WALL SECTION 4 IS THE REMAINDER OF THE WALL.
  2. COMPONENT AND CLADDING PRESSURES SHOWN ARE ULTIMATE PRESSURES AND CAN BE REDUCED BY 0.6 FOR ALLOWABLE PRESSURES.

UPLIFT CHARTS  
SCALE: 1/4" = 1'-0"



ROOF UPLIFT PLAN  
SCALE: 3/32" = 1'-0"

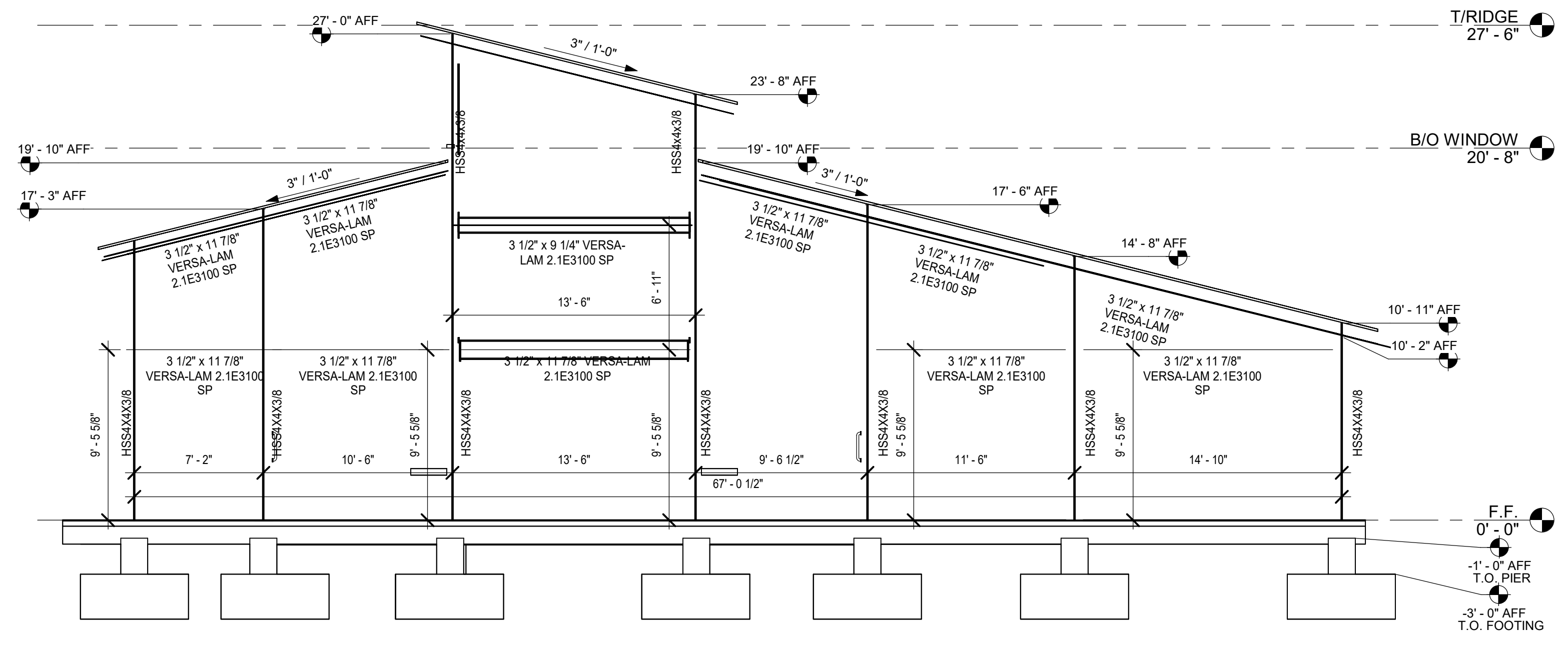
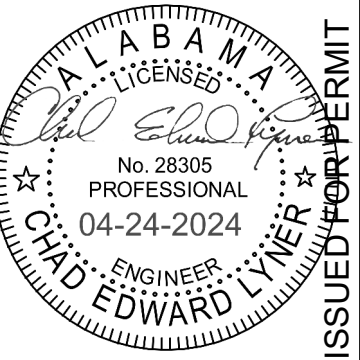
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107 St. Francis Street  
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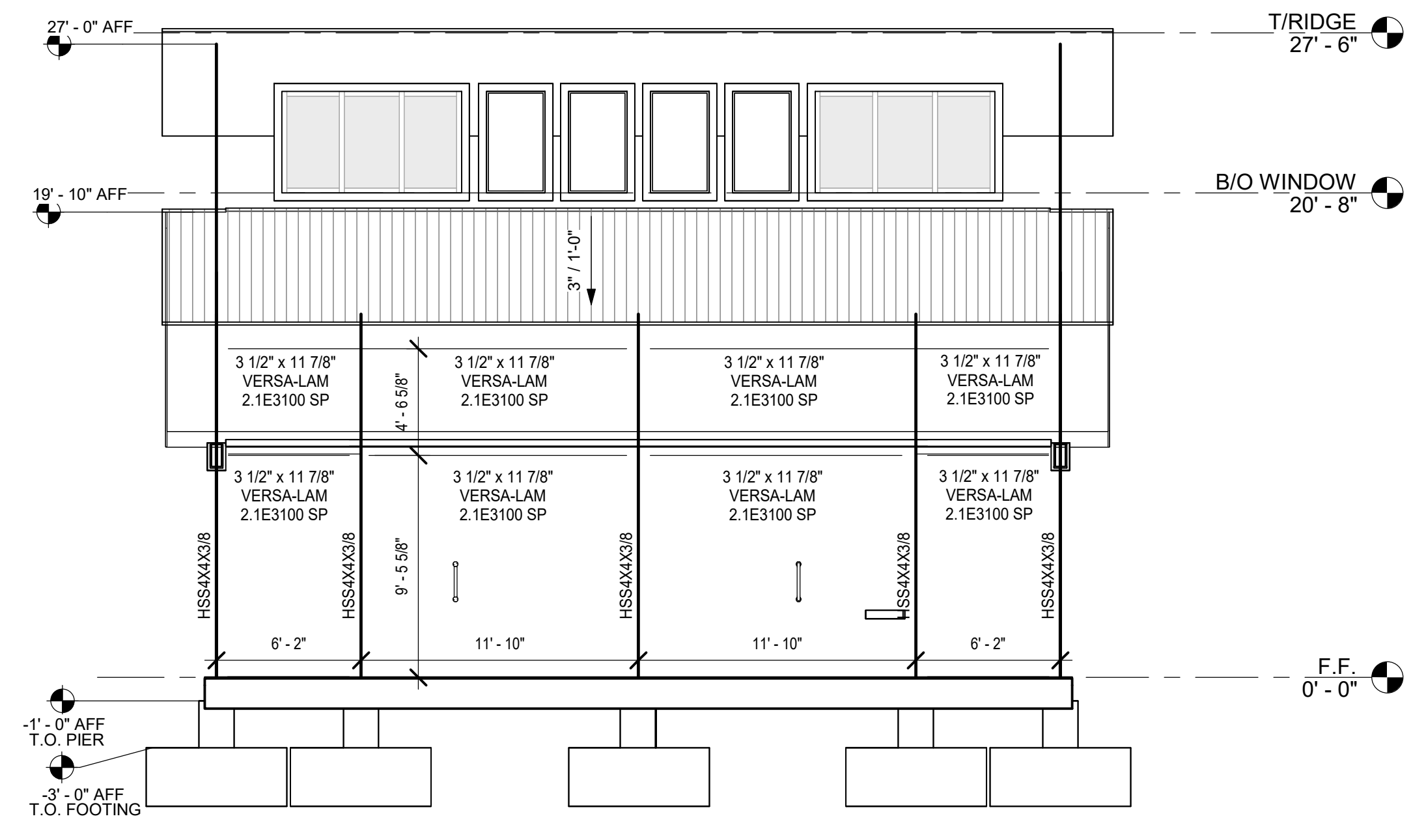
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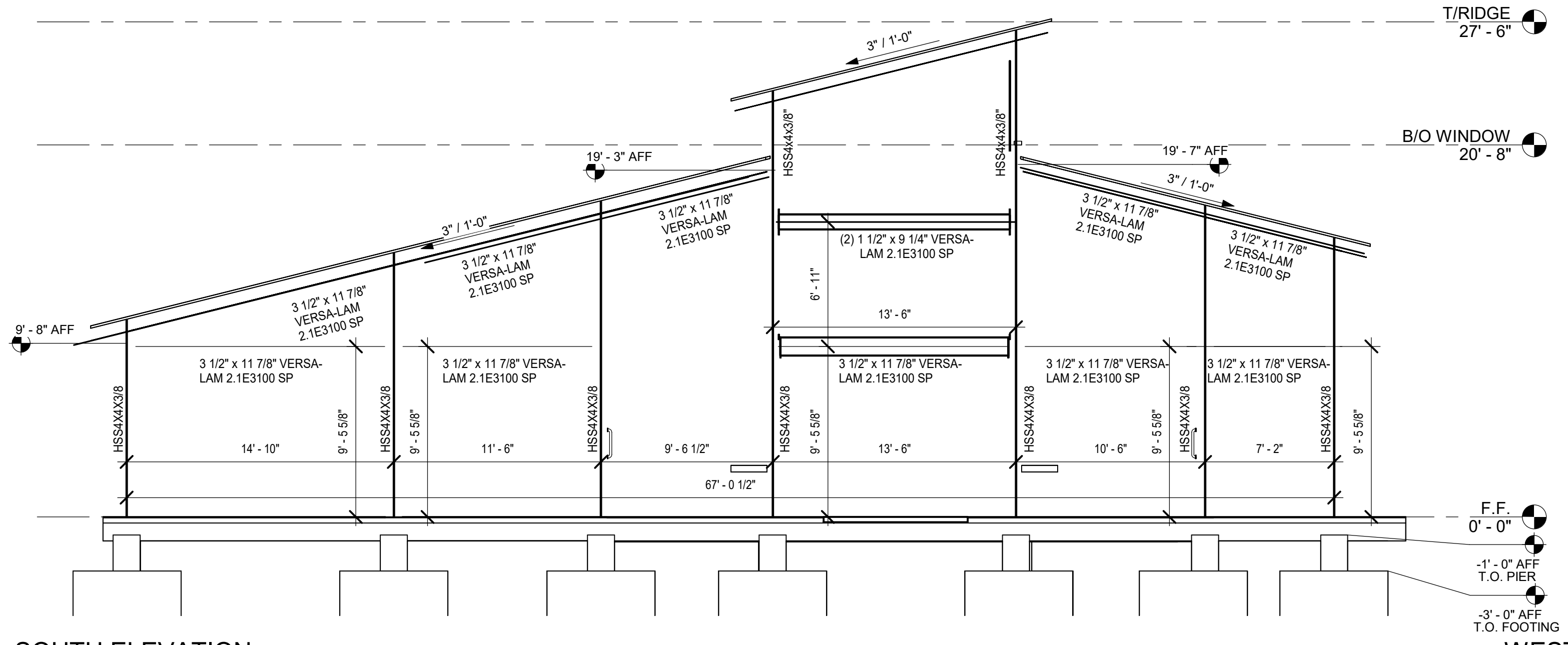
SCALE: 3/16" = 1'-0"  
DATE: May 5, 2024  
**S1.7**



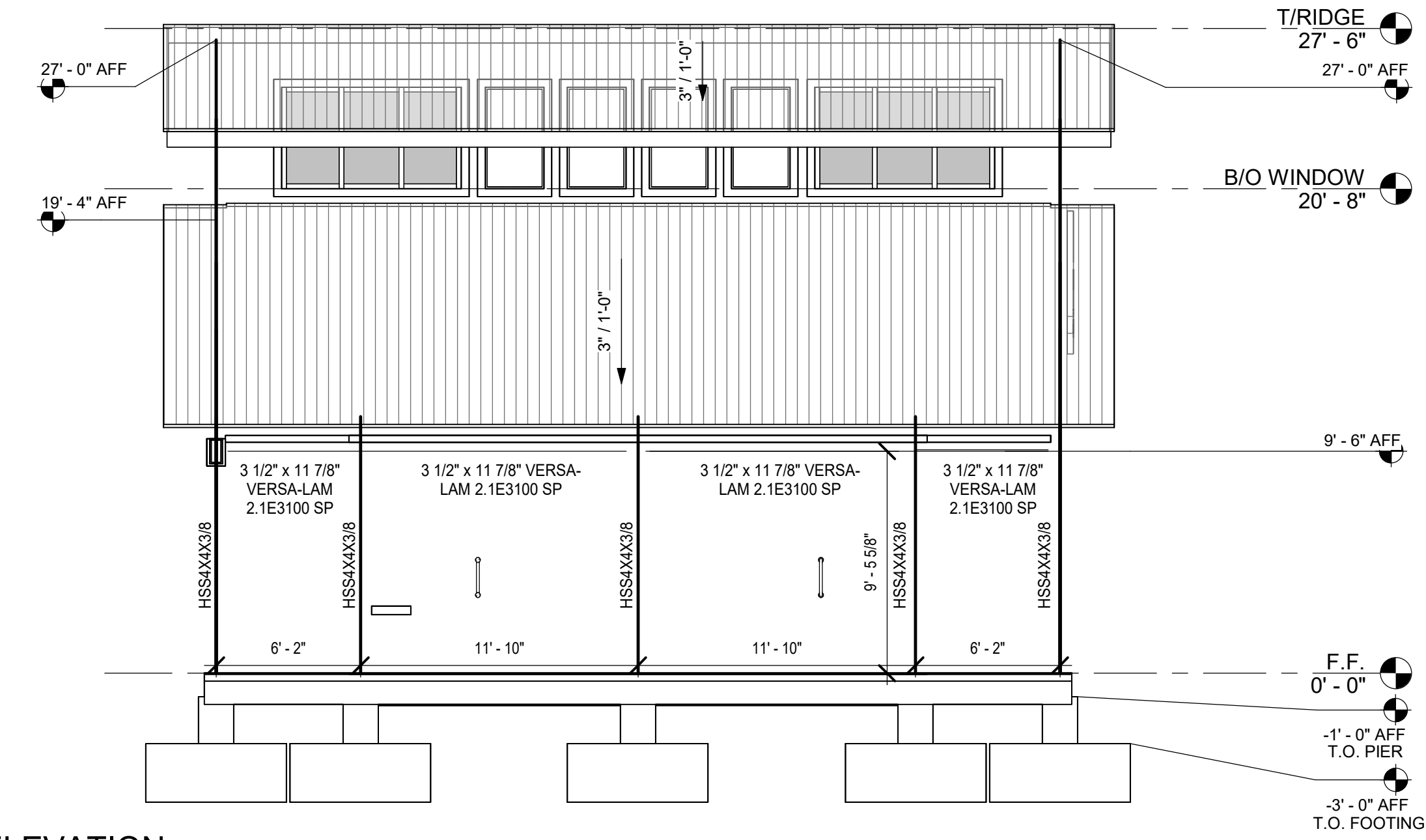
**NORTH ELEVATION**  
SCALE: 3/16" = 1'-0"



**EAST ELEVATION**  
SCALE: 3/16" = 1'-0"

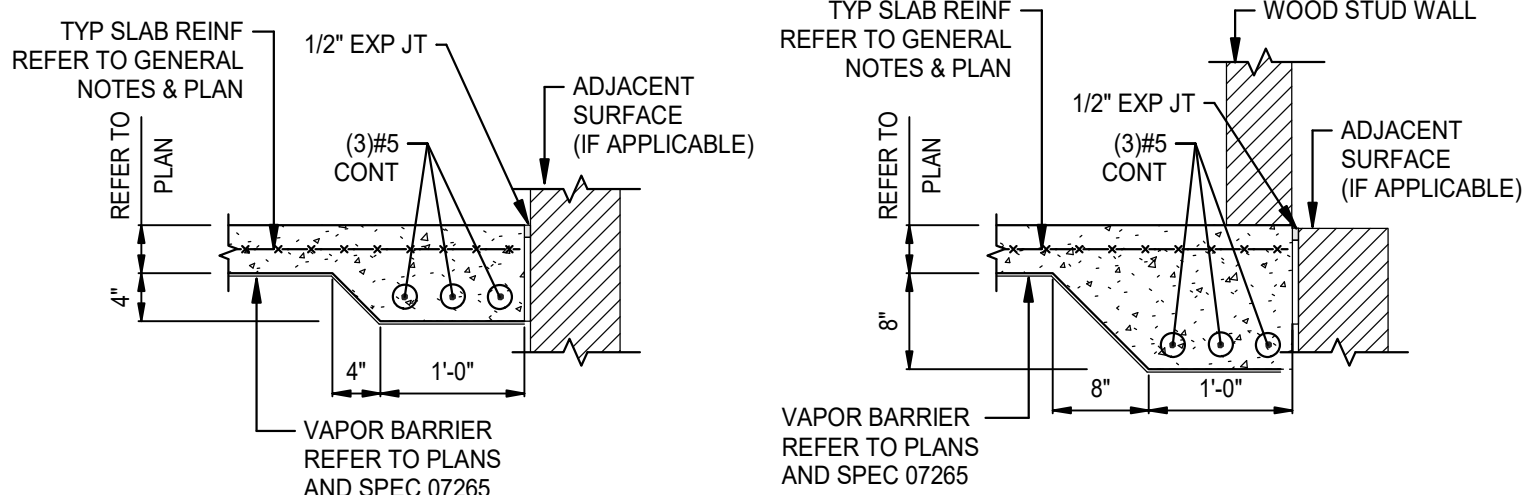


**SOUTH ELEVATION**  
SCALE: 3/16" = 1'-0"



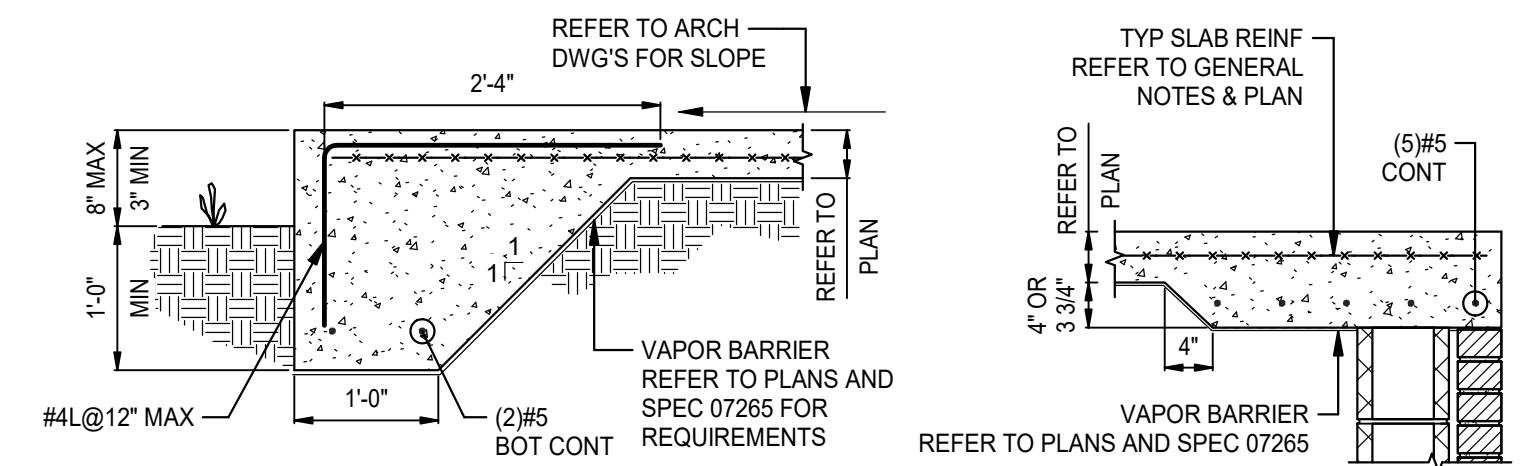
**WEST ELEVATION**  
SCALE: 3/16" = 1'-0"





**PORCH SLAB**

**INTERIOR SLAB**

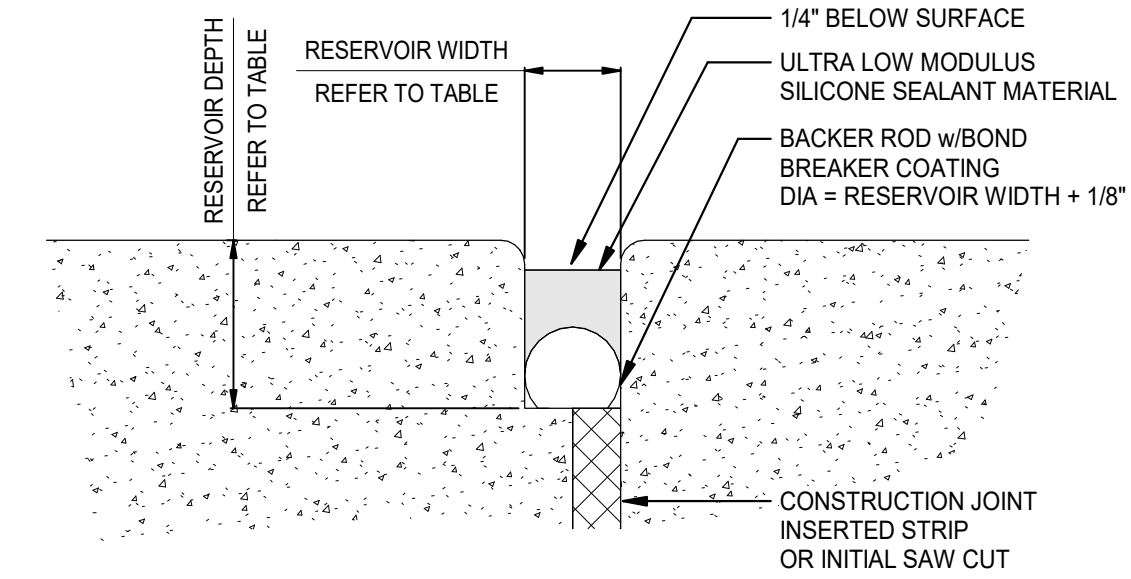
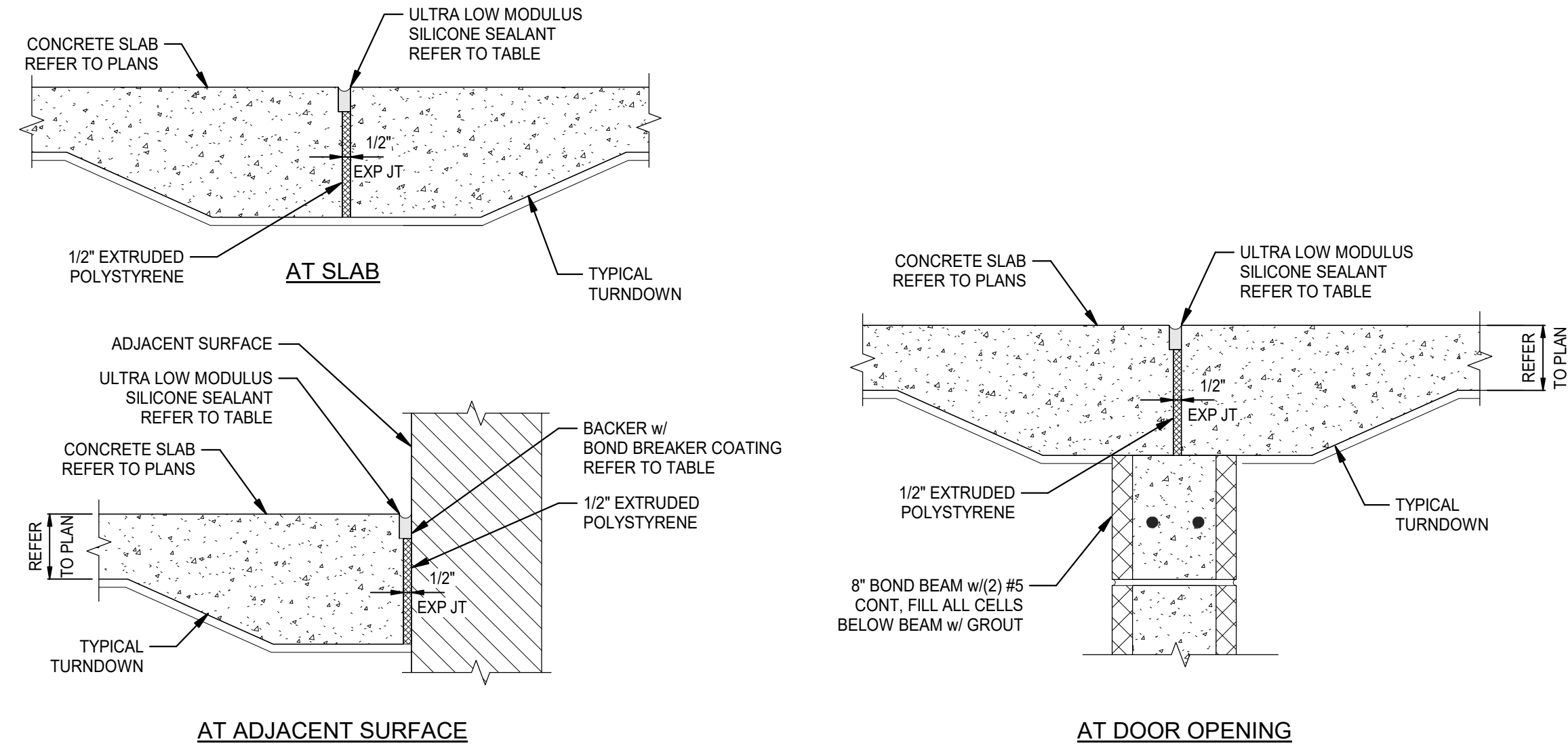


**EXTERIOR SLAB**

**SLAB OVER STEMWALL**

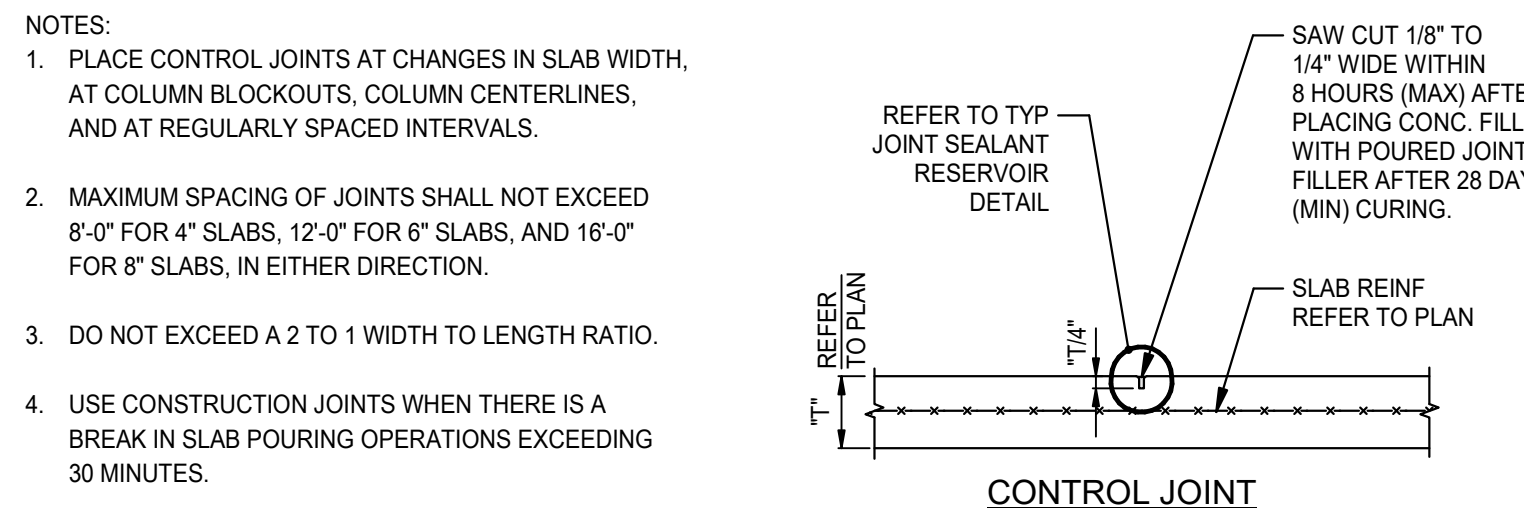
**1 TURNDOWN AT SLAB EDGE (WWF)**  
S3.1 3/4" = 1'-0"

**2 TYPICAL SLAB 1/2" EXPANSION JOINT (EJ)**  
S3.1 1 1/2" = 1'-0"



JOINT SPACING	SEALANT RESERVOIR SHAPE	
	WIDTH	DEPTH
15'-0" OR LESS	5/8"	3/4"
20'-0"	5/8"	3/4"
30'-0"	5/8"	3/4"
40'-0"	5/8"	1"

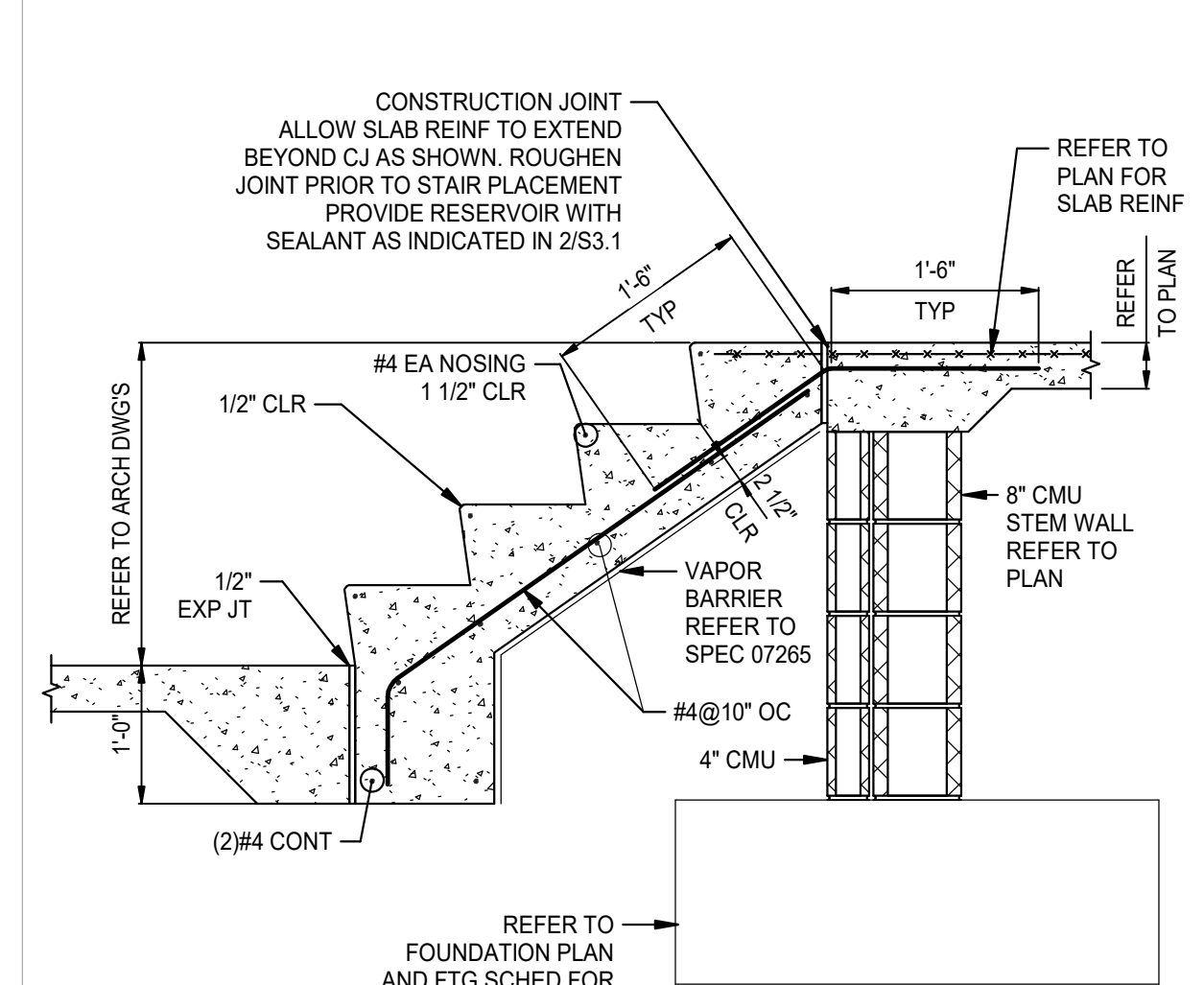
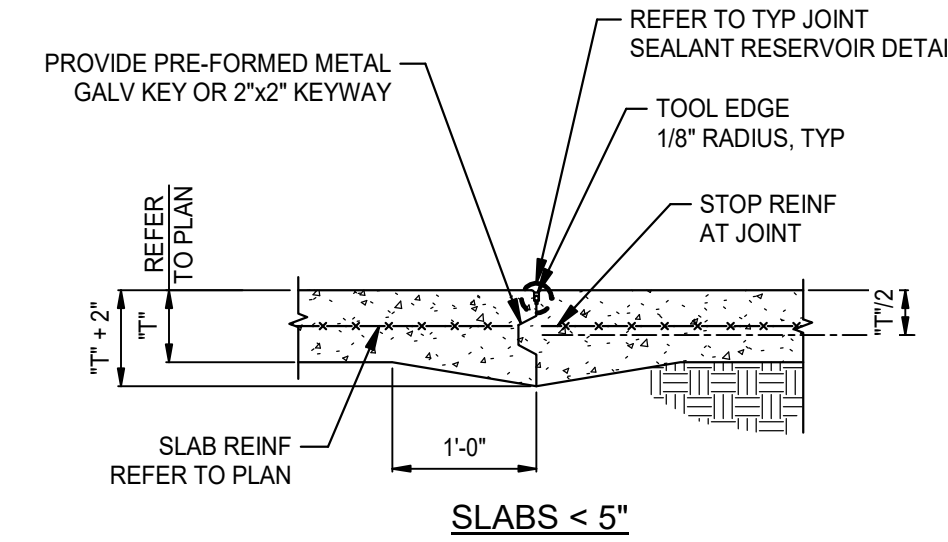
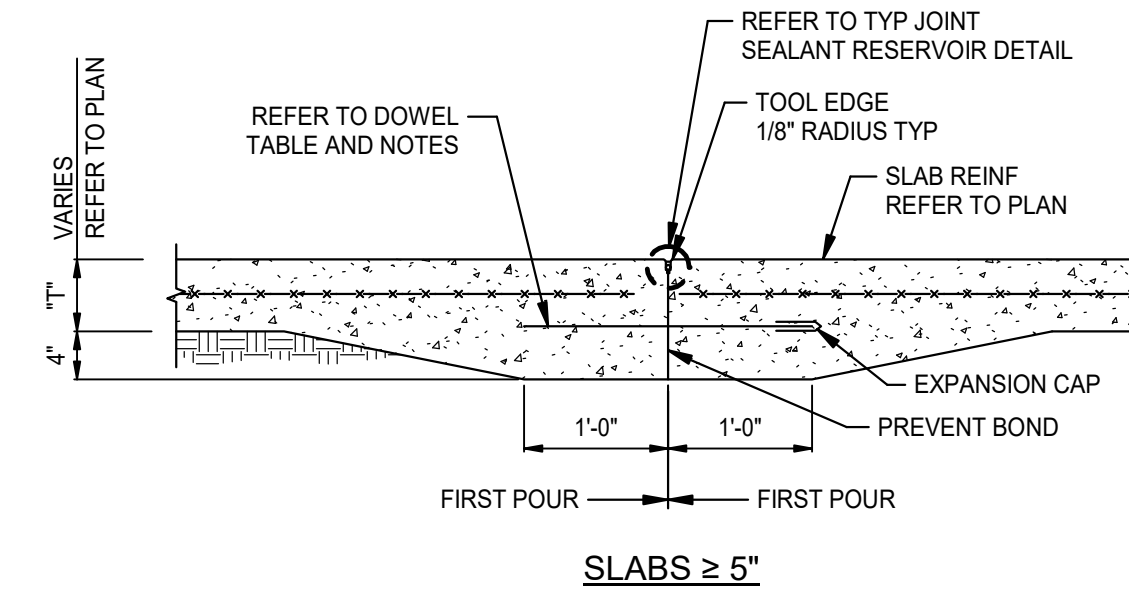
NOTE:  
SEALANT MATERIAL SHALL BE A FIELD MOLDED SEALANT OF ONE OF THE FOLLOWING TYPES:  
1. HOT APPLIED THERMOPLASTIC ASPHALT - RUBBER COMPOUNDS MEETING ASTM 1190.  
2. HOT Poured ELASTOMERIC TYPE SEALANTS - MEETING ASTM D3406.  
3. COLD APPLIED, MASTIC SINGLE OR MULTIPLE - COMPONENT SEALANTS MEETING ASTM D1850.



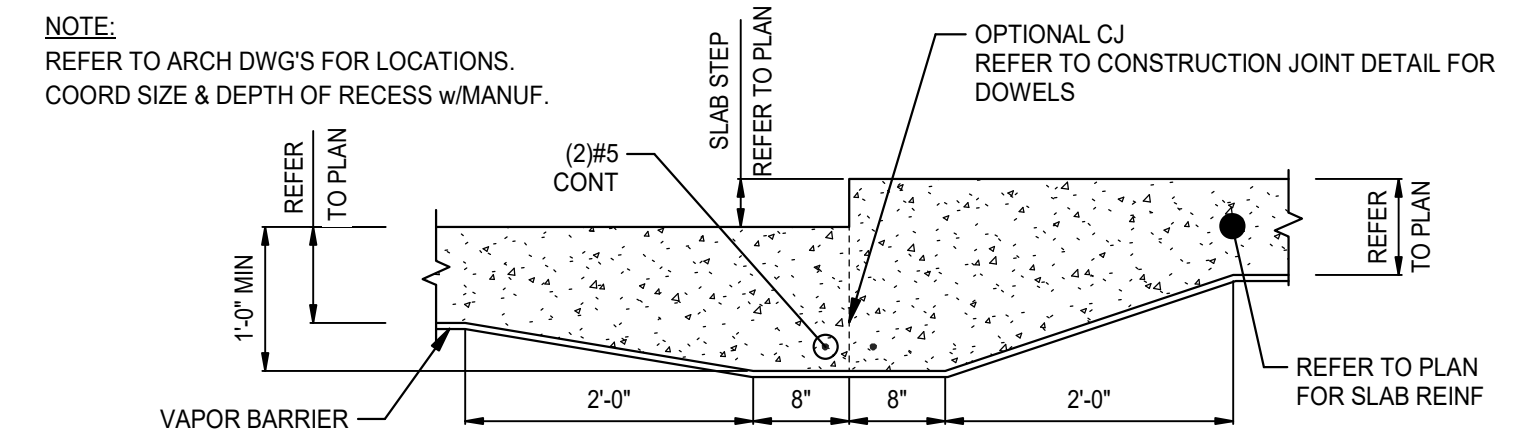
**3 TYPICAL SLAB ON GRADE CONTROL JOINT (CJ)**  
S3.1 3/4" = 1'-0"

DOWEL TABLE			
"T" SLAB DEPTH (INCHES)	DIAMETER (INCHES)	TOTAL LENGTH (INCHES)	CENTER TO CENTER SPACING (IN)
5	5/8	12	12
6	3/4	14	12
7	7/8	14	12
8	1	14	12
9	1 1/8	16	12
10	1 1/4	18	12
11	1 3/8	18	12
12	1 1/2	20	12

DOWEL NOTES:  
1. DOWELS SHALL BE PLAIN ROUND BARS EQUIVALENT TO ASTM A615 WITH A CORROSION RESISTANT COATING.  
2. ONE-HALF (1/2) OF EACH BAR SHALL BE COVERED WITH ONE COAT TAR. PLACE EXPANSION CAP ON COATED SIDE.  
3. DOWELS SHALL BE PLACED PARALLEL TO THE CENTERLINE AND SURFACE OF THE SLAB. TOLERANCE OF THE PLACEMENT SHALL BE ±1/4".

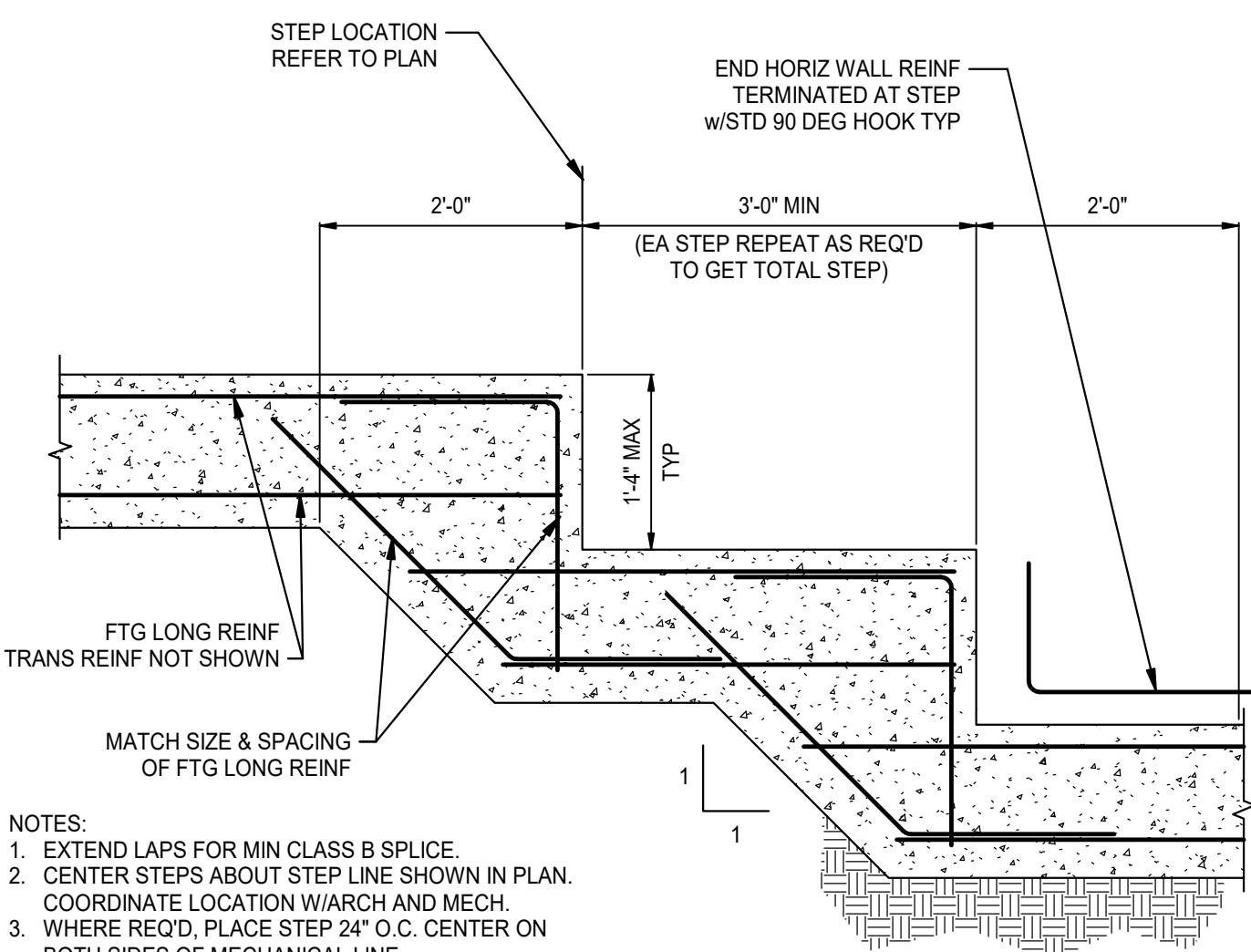


**6 TYPICAL CONCRETE STAIRS**  
S3.1 3/4" = 1'-0"

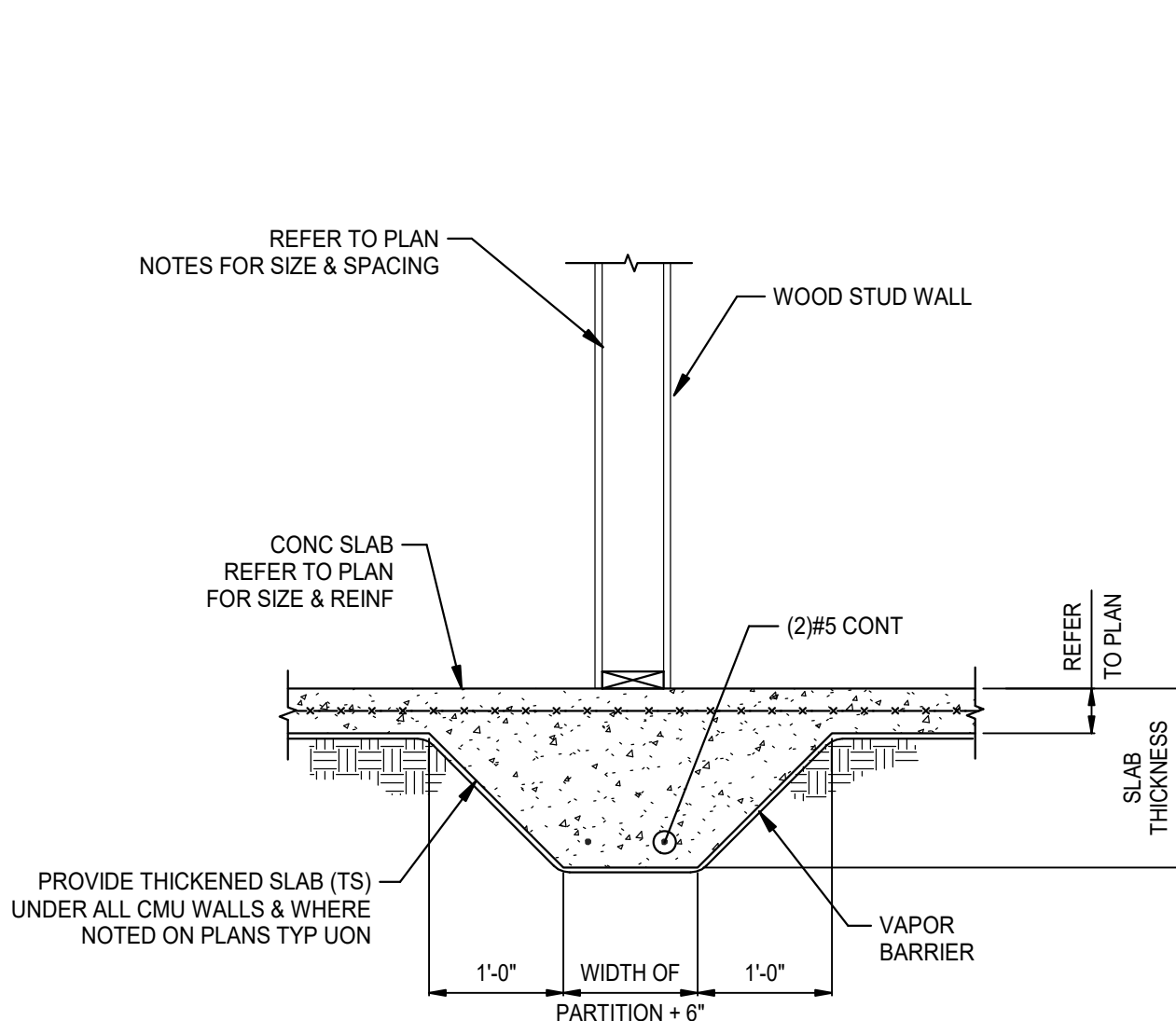


**4 TYPICAL SLAB ON GRADE STEP OR RECESS**  
S3.1 3/4" = 1'-0"

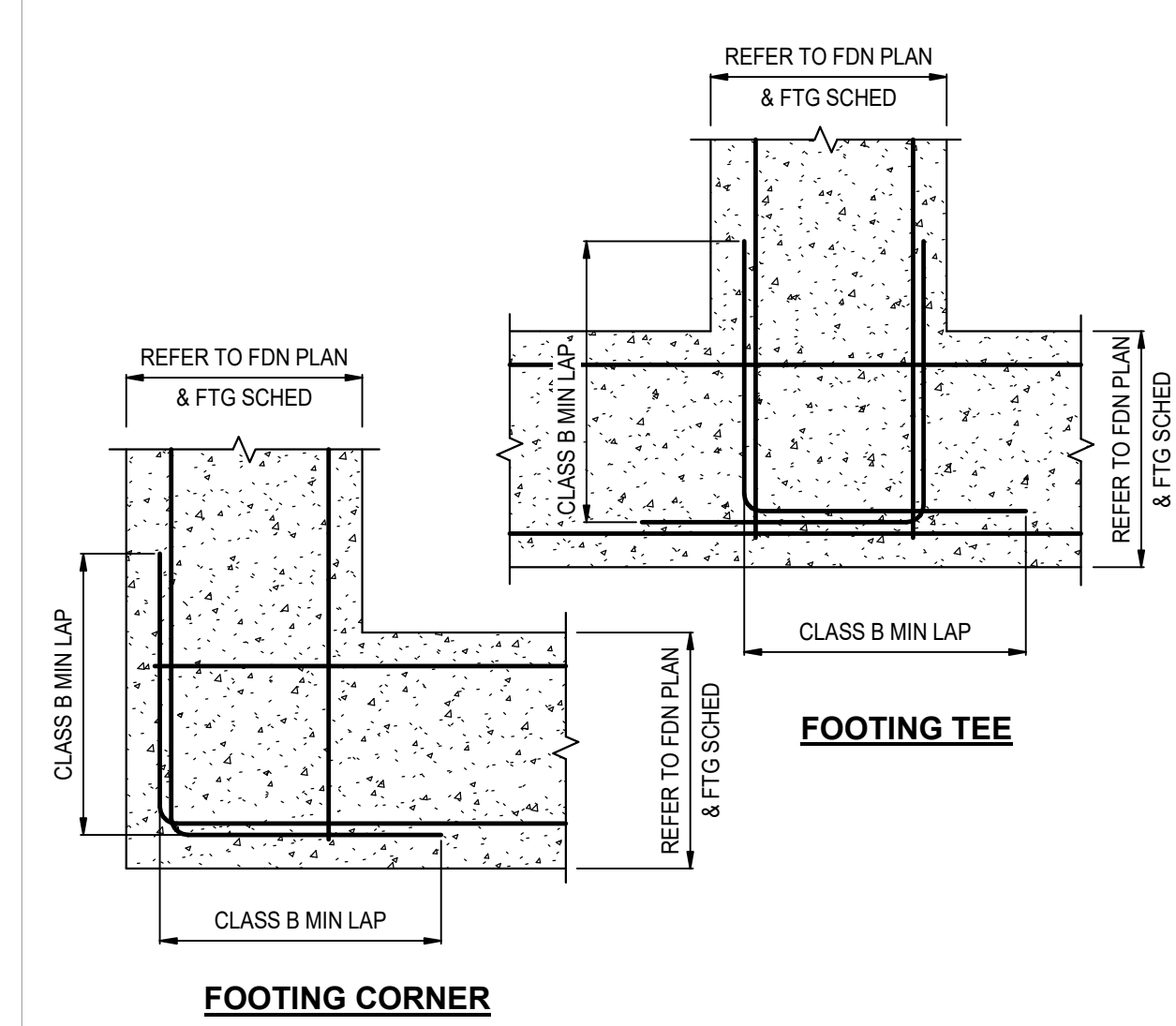
**5 TYPICAL SLAB ON GRADE CONSTRUCTION JOINT**  
S3.1 3/4" = 1'-0"



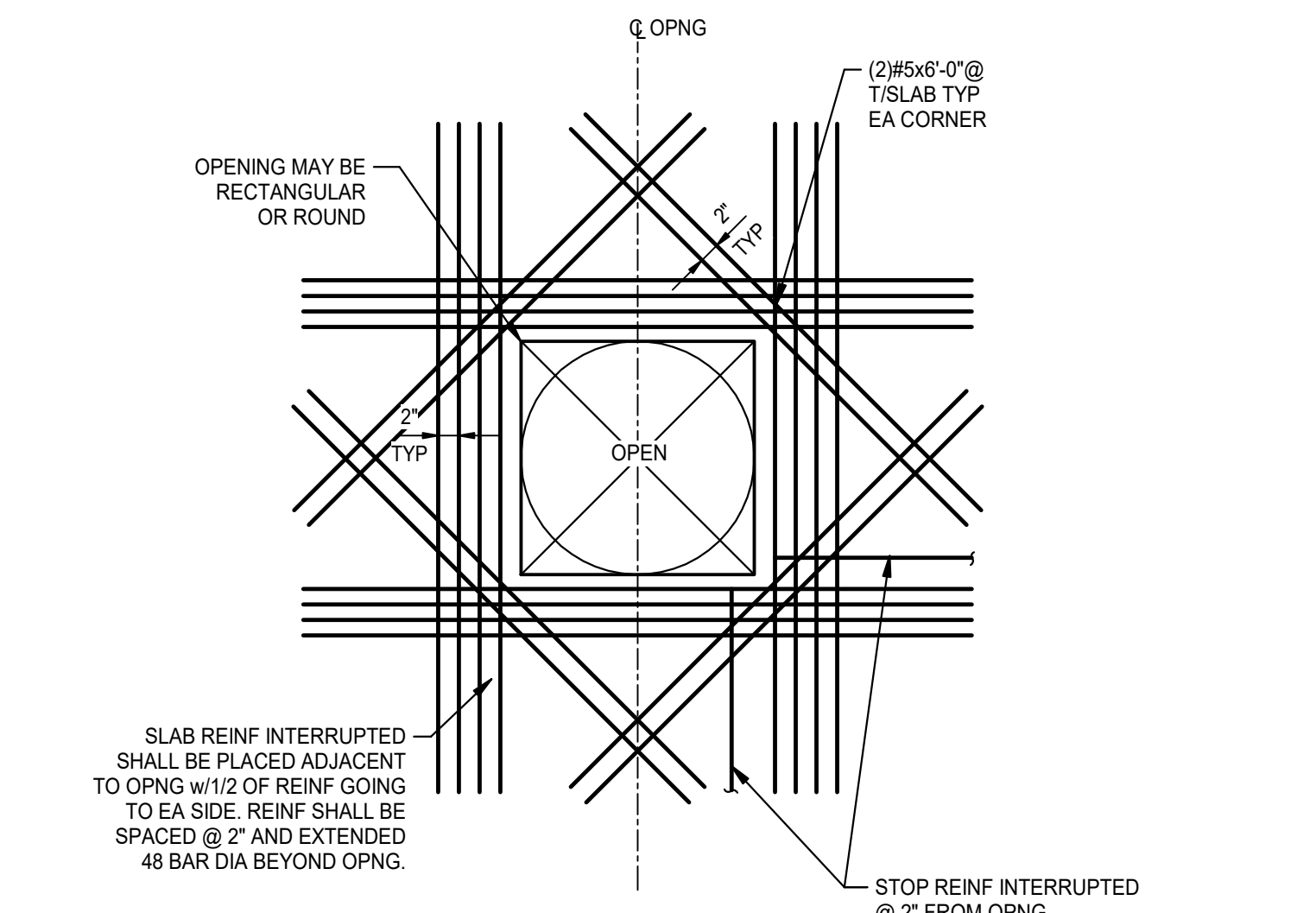
**8 TYPICAL FOOTING STEPS**  
S3.1 3/4" = 1'-0"



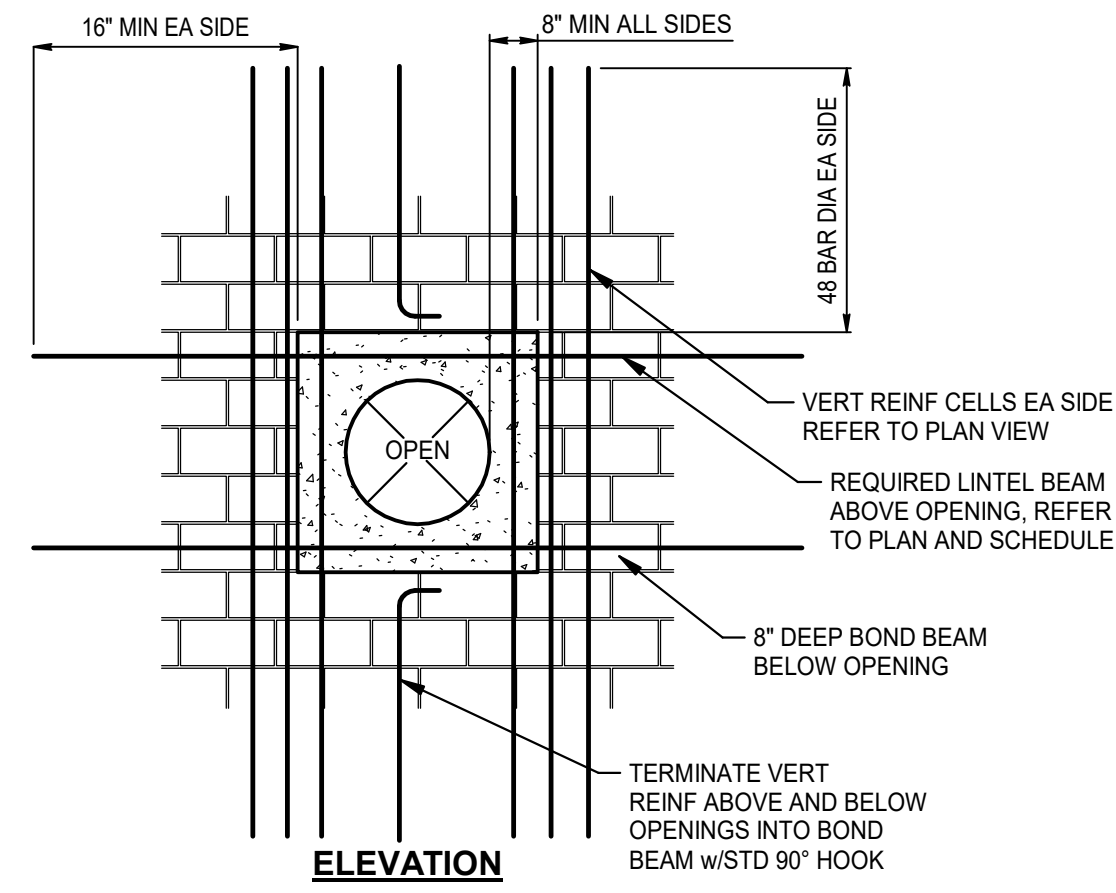
**9 TYPICAL THICKENED SLAB (TS) UNDER PARTITION WALL**  
S3.1 3/4" = 1'-0"



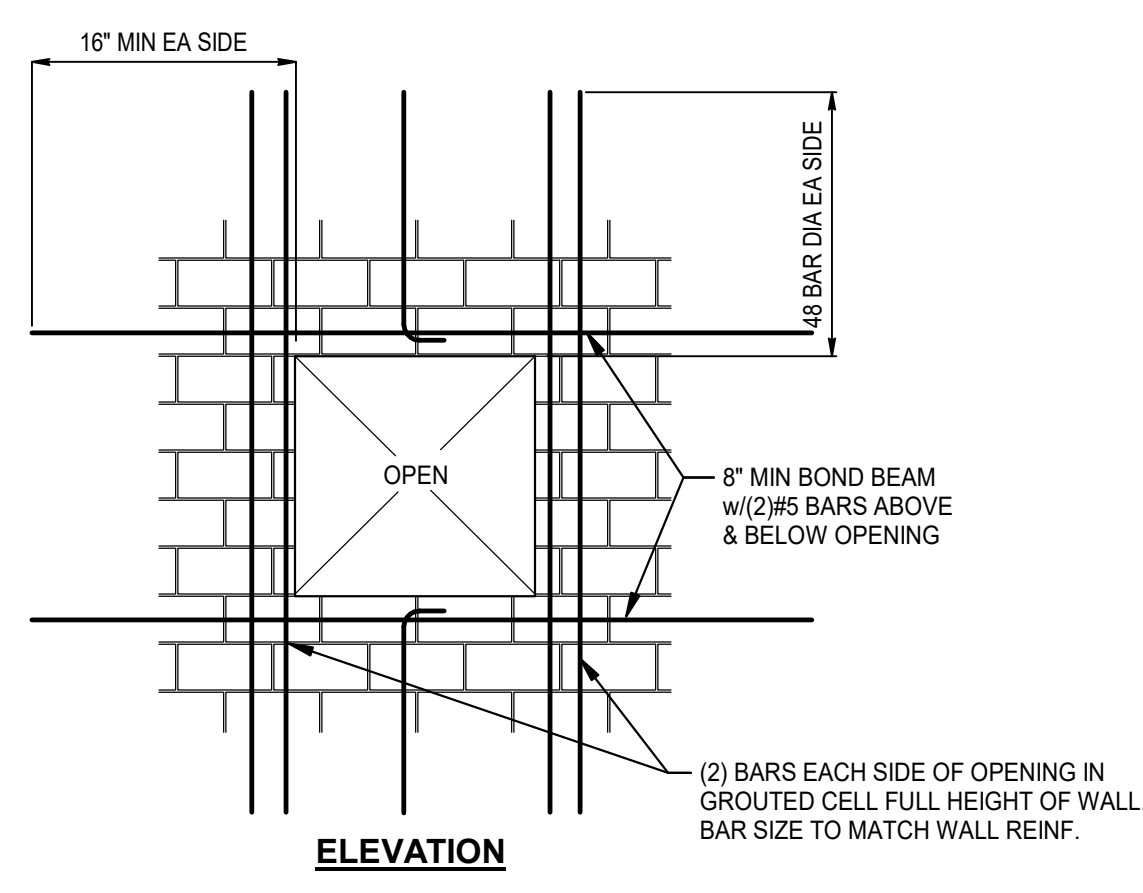
**10 REINFORCING AT FOOTING TEES AND CORNERS**  
S3.1 3/4" = 1'-0"



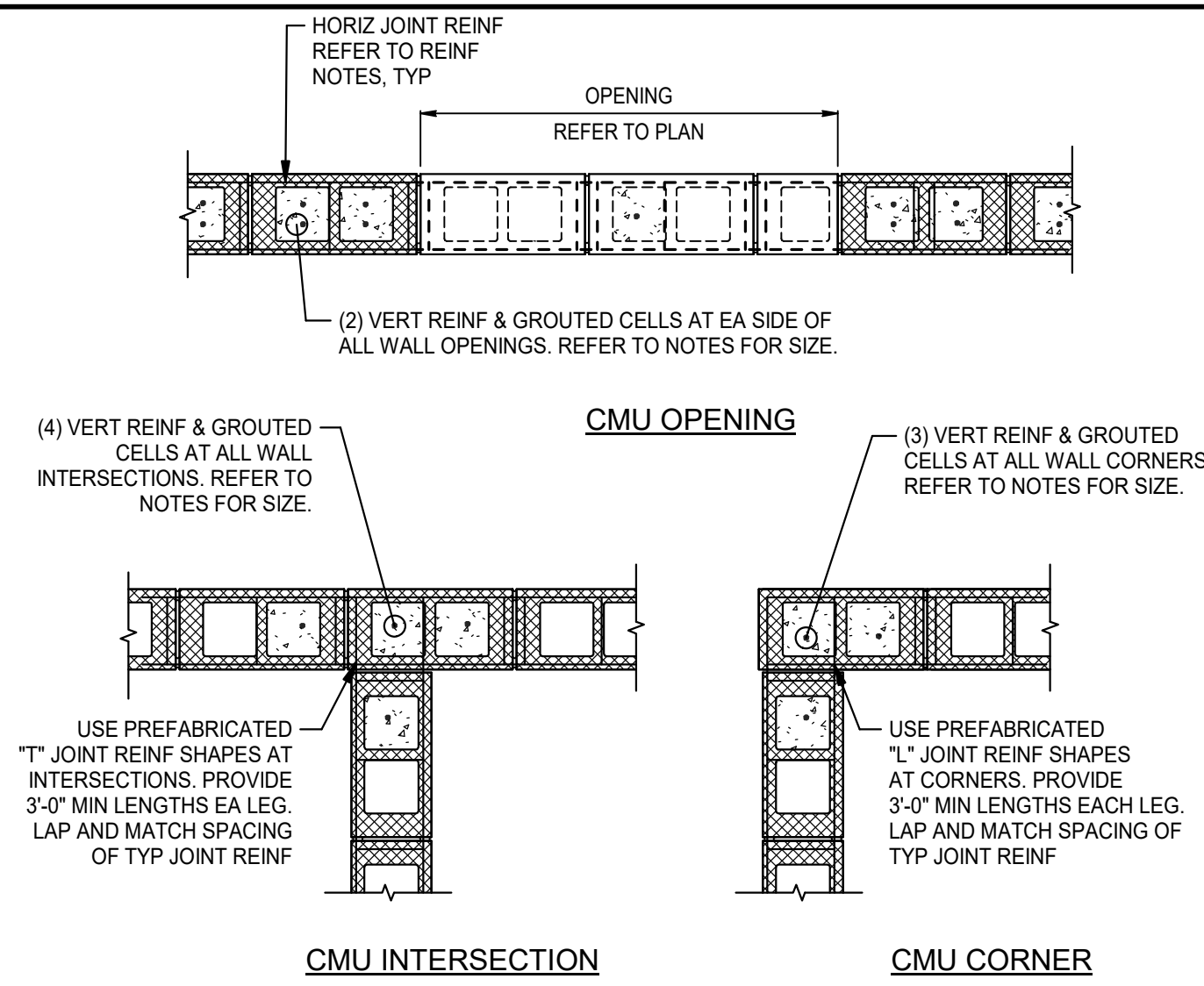
**7 ADDITIONAL REINF AT SLAB OPENING**  
S3.1 3/4" = 1'-0"



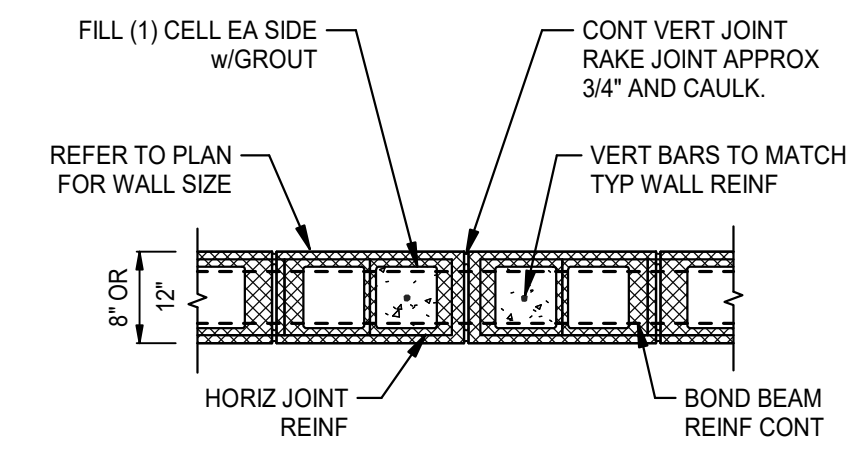
**1** CIRCULAR OPENING IN MASONRY WALL  
S3.2 3/4" = 1'-0"



**2** SQUARE OPENING IN MASONRY WALL  
S3.2 3/4" = 1'-0"

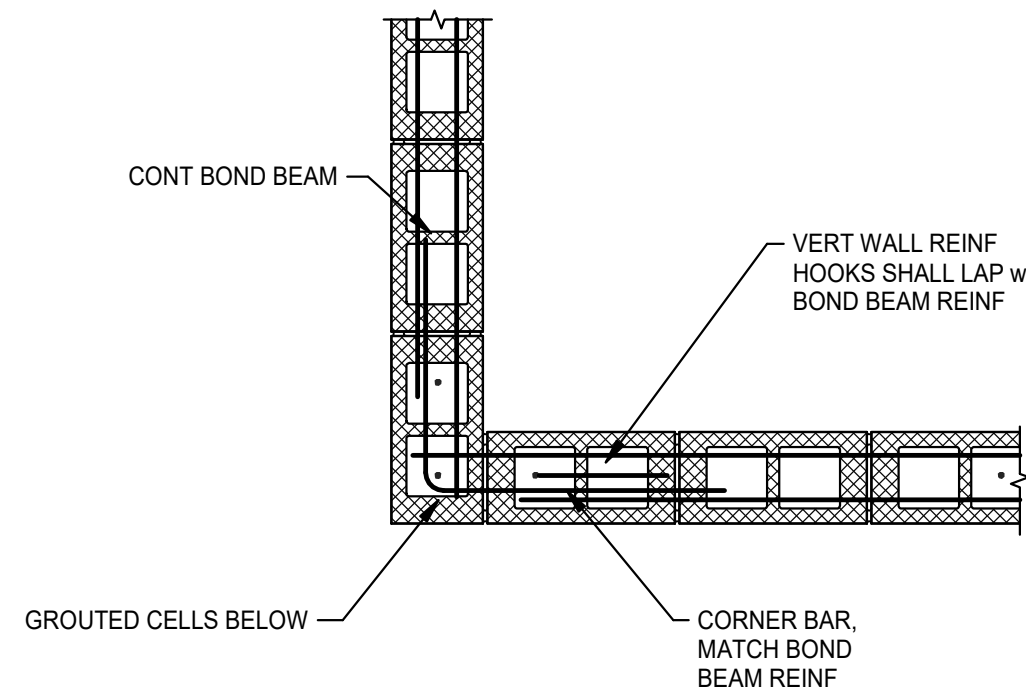


**3** TYPICAL CMU DETAILS  
S3.2 3/4" = 1'-0"



- NOTES:
1. CMU CONTROL JOINTS ARE NOT TO EXCEED 25'-0", TYPICAL UNLESS OTHERWISE NOTED.
  2. THE JOINTS SHALL BE LOCATED A MINIMUM OF 24" FROM DOOR OR WINDOW OPENINGS TO MISS LINTELS.
  3. THE TYPICAL HORIZONTAL JOINT REINFORCING SHALL BE TERMINATED 2" FROM EACH SIDE OF JOINT. ALL BOND BEAM REINFORCING SHALL CONTINUE THRU THE JOINT.
  4. WALL SEGMENTS THAT EXCEED 25' AND DO NOT CONTAIN CONTROL JOINTS MUST BE REINFORCED WITH TYPICAL HORIZONTAL BOND BEAMS SPACED AT 5'-0" MAXIMUM VERTICAL.

**4** TYPICAL CMU CONTROL JOINT DETAIL  
S3.2 3/4" = 1'-0"



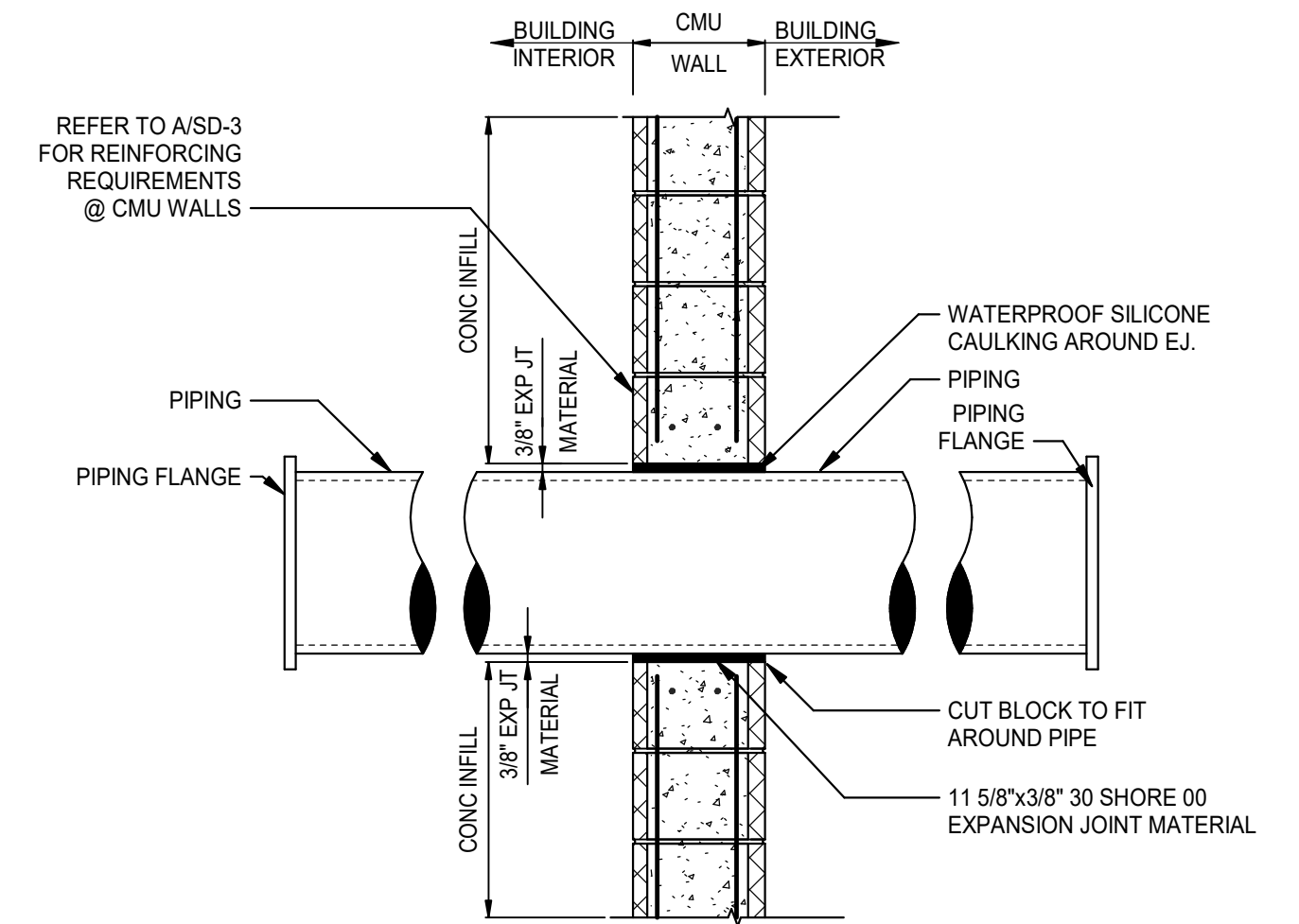
- NOTES:
1. ALL LAPS SHALL BE 48 BAR DIA MINIMUM.
  2. SIMILAR FOR BOND BEAMS WITH (1) HORIZ BAR

**5** BOND BEAM CORNER REINFORCING DETAIL  
S3.2 3/4" = 1'-0"

8" & 12" EXTERIOR CMU JAMB SCHEDULE		8" INTERIOR CMU JAMB SCHEDULE	
OPENING SIZE "W"	JAMB WIDTH & REINF	OPENING SIZE "W"	JAMB WIDTH & REINF
3'-4"	8" w/(2) #5 EA CELL	3'-4"	8" w/(1) #5 EA CELL
≤ 7'-4"	16" w/(2) #5 EA CELL	≤ 7'-4"	16" w/(1) #5 EA CELL
≤ 12'-0"	24" w/(2) #5 EA CELL	≤ 11'-8"	16" w/(2) #5 EA CELL

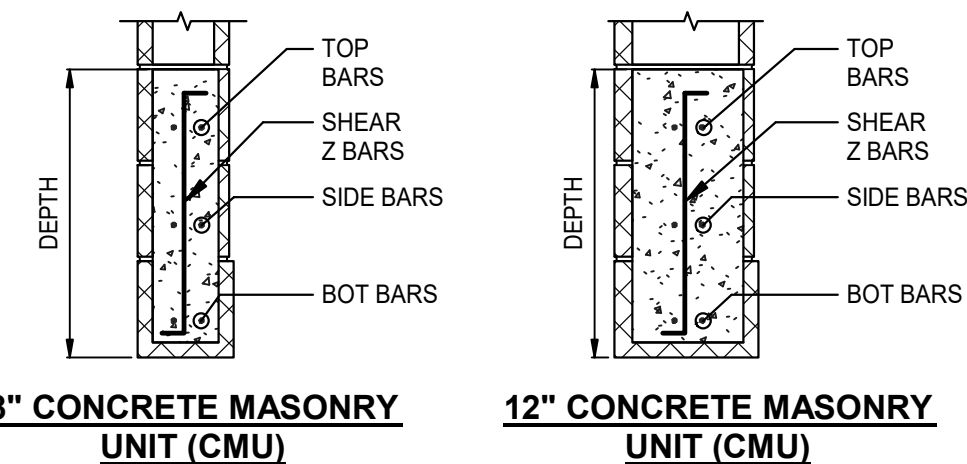
- NOTE:
1. REFER TO DETAIL E/SD-4 FOR REINF PLACEMENT IN DOUBLE REINFORCED CELLS ((2) BARS EA CELL).
  2. REFER TO CMU WALL NOTES FOR TYPICAL REINFORCING
  3. APPLIES TO ALL OPENINGS INCLUDING BUT NOT LIMITED TO DOORS, WINDOWS, LOUVERS, DUCT PENETRATIONS, ETC.

**6** TYPICAL CMU JAMB SCHEDULE  
S3.2 3/4" = 1'-0"



**7** TYPICAL PIPING PENETRATION DETAIL @ EXTERIOR OF CMU  
S3.2 3/4" = 1'-0"

CMU LINTEL DESIGNATION						
WALL TYPE	WALL SIZE	LINTEL SPAN				
		≤ 3'-4"	7'-4"	11'-8"	14'-8"	18'-0"
INTERIOR	8" CMU	1	2	3	4	4
EXTERIOR	12" CMU	5	6	7	8	8



**8** TYPICAL LINTEL SCHEDULE  
S3.2 3/4" = 1'-0"

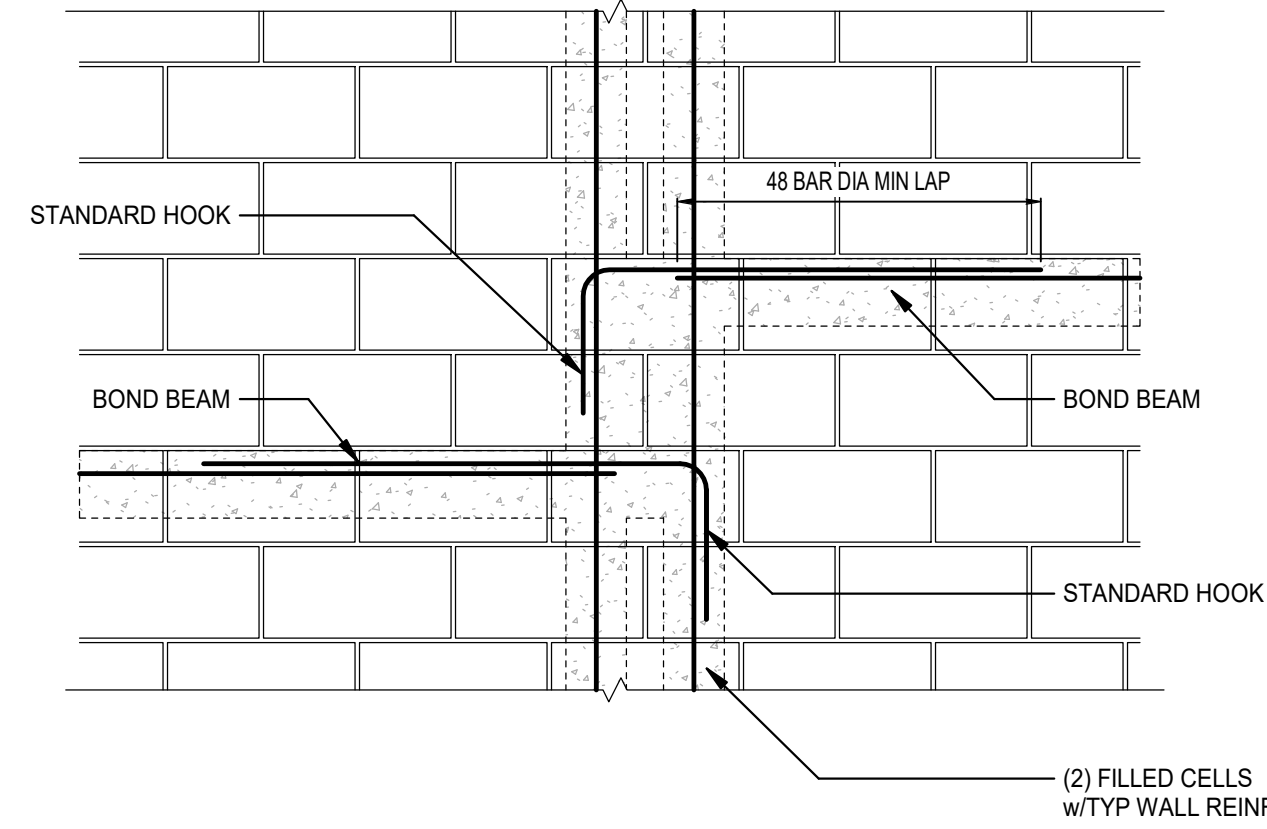
LINTEL BEAM SCHEDULE						
DESIGNATION	SIZE (WIDTH x DEPTH)	REINFORCING				SHEAR
		TOP BARS	BOT BARS	SIDE BARS	SHEAR	
1	8"x8" CMU	-	(2)#5	-	-	-
2	8"x16" CMU	(2)#5	(2)#5	-	-	-
3	8"x24" CMU	(2)#5	(2)#5	(2)#5	-	-
4	8"x32" CMU	(2)#5	(2)#5	(4)#5	-	-
5	12"x8" CMU	-	(2)#6	-	-	-
6	12"x16" CMU	(2)#6	(2)#6	-	-	-
7	12"x24" CMU	(2)#6	(2)#6	(2)#6	-	-
8	12"x32" CMU	(2)#6	(2)#6	(2)#6	-	-

- LINTEL NOTES:
1. PROVIDE 16" MINIMUM BEARING FOR CMU AND CONCRETE LINTELS.
  2. SHORE LINTELS UNTIL WALL ABOVE IS COMPLETED AND ALL CELLS ARE FILLED.
  3. EXTEND TOP & BOT REINF 48 BAR DIA MIN. BEYOND JAMB @ EACH END OR TERMINATE W/STD HOOK.
  4. BOTTOM BLOCK SHALL BE SOLID FACE LINTEL BLOCK WHEN FINISH BLOCK IS EXPOSED.

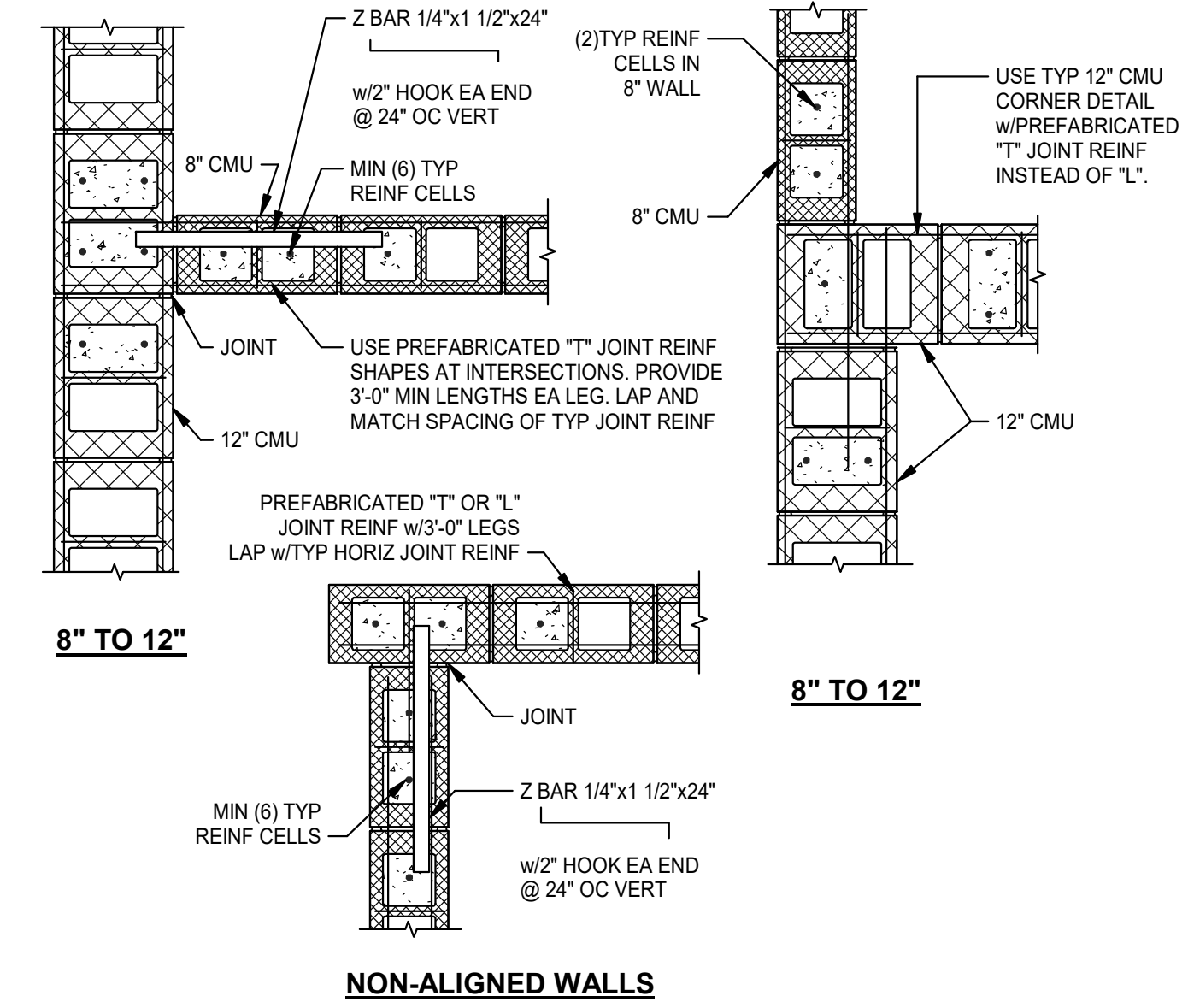


1. REFER TO SHEET S0.1 AND S0.2 FOR ADDITIONAL NOTES.
2. DOWEL ALL CMU REINFORCEMENT IN FOOTINGS AND EXTEND INTO UPPERMOST BOND BEAM WITH 90 DEGREE HOOKS.
3. FILL ALL CELLS CONTAINING REINFORCEMENT AND ADDITIONAL CELLS AS INDICATED WITH 3,000 PSI PEA GRAVEL CONCRETE.
4. PROVIDE FOUR (4) FILLED CELLS OF TYPICAL WALL REINFORCING AT INTERSECTIONS, (3) FILLED CELLS OF TYPICAL WALL REINFORCING AT CORNERS, AND TWO (2) FILLED CELLS OF TYPICAL WALL REINFORCING AT EACH SIDE OF OPENINGS AND ENDS OF WALLS. PROVIDE (5) FILLED CELLS OF TYPICAL WALL REINFORCING AT CORNERS OF STAIRWELL AND ELEVATOR WALLS, UNLESS OTHERWISE NOTED.
5. FOR REINFORCEMENT ADJACENT TO INTERIOR CMU WALL OPENINGS, COORDINATE WITH JAMB SCHEDULE SHOWN ON THIS SHEET.
6. ALL CONCRETE MASONRY UNITS SHALL BE PLACED IN RUNNING BOND.
7. TYPICAL 8" CMU WALL REINFORCEMENT:
  - A. REINFORCE WITH VERTICAL BARS: #5 @ 24" ON CENTER WITH ADDITIONAL REINFORCING AS INDICATED IN NOTE 4.
  - B. PROVIDE 16" CMU BOND BEAM WITH (2)#5 CONTINUOUS REINFORCING BARS AT TOP OF ALL WALLS AND AT ROOF.
  - C. PLACE THE REINFORCING IN THE CENTER OF THE WALL, UNLESS OTHERWISE NOTED.
8. TYPICAL 12" CMU WALL REINFORCEMENT:
  - A. DOUBLE REINFORCE WITH VERTICAL BARS #5@24" ON CENTER WITH ADDITIONAL REINFORCING AS INDICATED IN NOTE 4.
  - B. PROVIDE 16" CMU BOND BEAM WITH (2)#5 CONTINUOUS AT TOP OF ALL WALLS.
  - C. PLACE THE REINFORCEMENT SO CENTERLINE OF REINFORCING IS 1 1/2" OFF INTERIOR FACE OF CMU CAVITIES.
9. HORIZONTAL JOINT REINFORCING IN ALL BLOCK WALLS SHALL BE STANDARD (9GA SIDE AND CROSS RODS) LADDER TYPE WALL REINFORCING @ 16". ALL WALLS PERPENDICULAR TO EXTERIOR WALLS SHALL HAVE ADDITIONAL PREFABRICATED "T" OR "L" JOINT REINFORCING AS INDICATE IN TYPICAL CMU DETAILS.
10. GROUT STOP SHALL BE A FIBERGLASS MESH CONFORMING TO ASTM STANDARD D1688-73, TYPE 207.
11. SPLICE ALL BARS 48 BAR DIAMETER, UNLESS OTHERWISE NOTED.
12. USE (1) TOP & BOTTOM CORNER BAR (MATCH TYPICAL REINFORCING) WITH 48 BAR DIAMETER LONG LEGS EACH WAY IN ALL BOND BEAM CORNERS & INTERSECTIONS. PLACE AT EXTERIOR FACE, UNLESS OTHERWISE NOTED.
13. THE LOWEST VERTICAL BAR IN ALL BLOCK WALLS SHALL HOOK 90 DEGREES INTO THE FOOTING OR SLAB WITH A MINIMUM 8" LEG UNLESS THE VERTICAL REINFORCING PASSES THRU THE SLAB TO A CONTINUOUS WALL BELOW.
14. THE HIGHEST VERTICAL BAR IN ALL BLOCK WALLS SHALL HOOK 90 DEGREES INTO THE UPPERMOST BOND BEAM WITH A MINIMUM 8" LEG UNLESS THE VERTICAL REINFORCING PASSES THRU THE SLAB TO A CONTINUOUS WALL ABOVE. IF THE WALL IS CAPPED WITH A SLAB, EXTEND 90 DEGREE HOOKS INTO THE SLAB AND LAP WITH THE VERTICAL WALL REINFORCING.
15. REFER TO DETAILS B AND C FOR ADDITIONAL REINFORCING AT WALL OPENINGS. OPENINGS LESS THAN 8"x8" OR 8" DIAMETER SHALL BE EXEMPT FROM THIS REQUIREMENT PROVIDED THAT ANY PORTION OF OPENING IS NOT LOCATED WITHIN A REINFORCED CELL.
16. CONDUIT PLACED IN REINFORCED CELLS SHALL BE LIMITED TO (1) CONDUIT PER REINFORCED OR FILLED CELL. MAXIMUM CONDUIT SIZE SHALL NOT EXCEED 1" OUTSIDE DIAMETER.

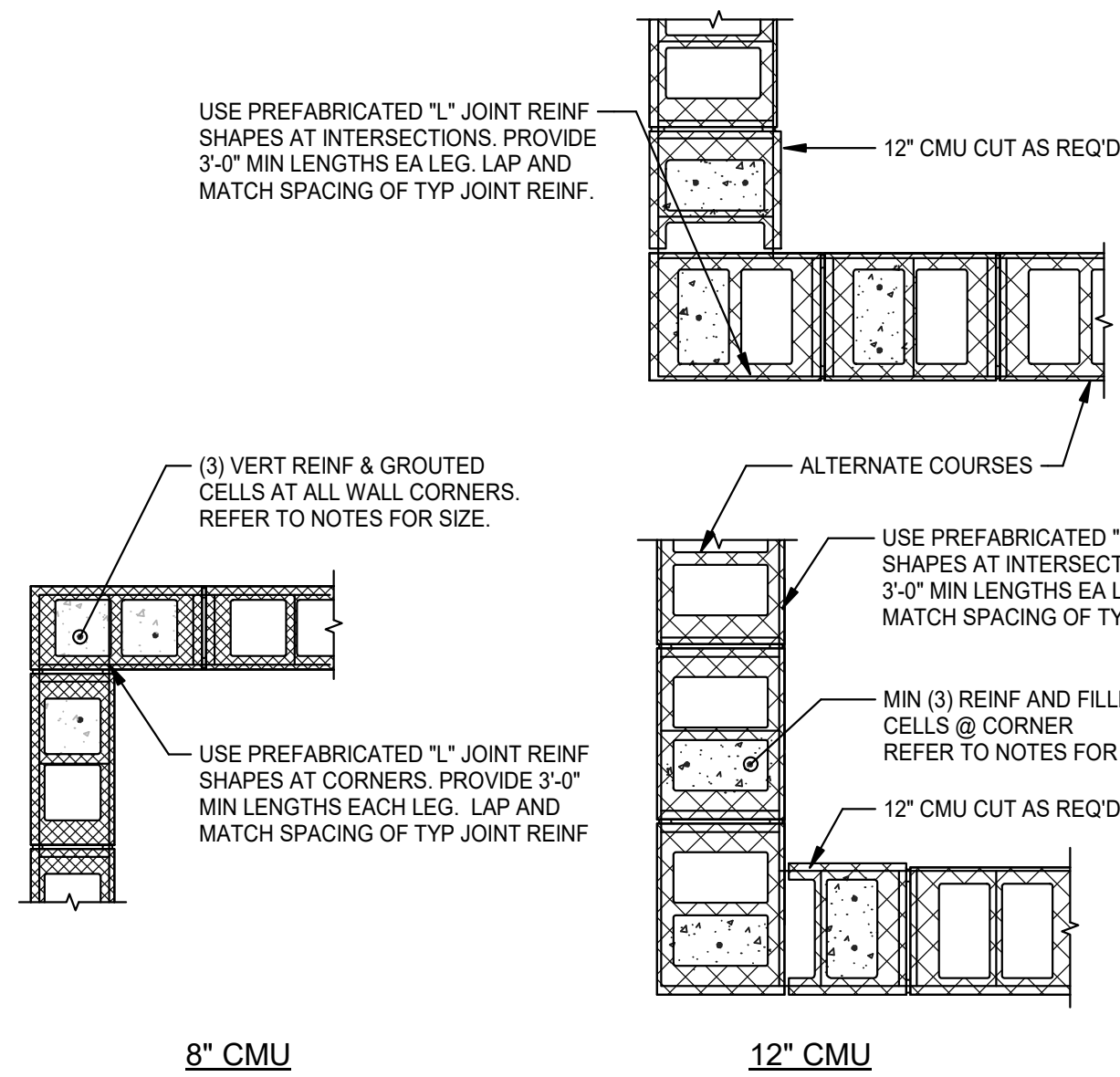
**1** TYPICAL CMU WALL REINFORCING NOTES  
S3.3 3/4" = 1'-0"



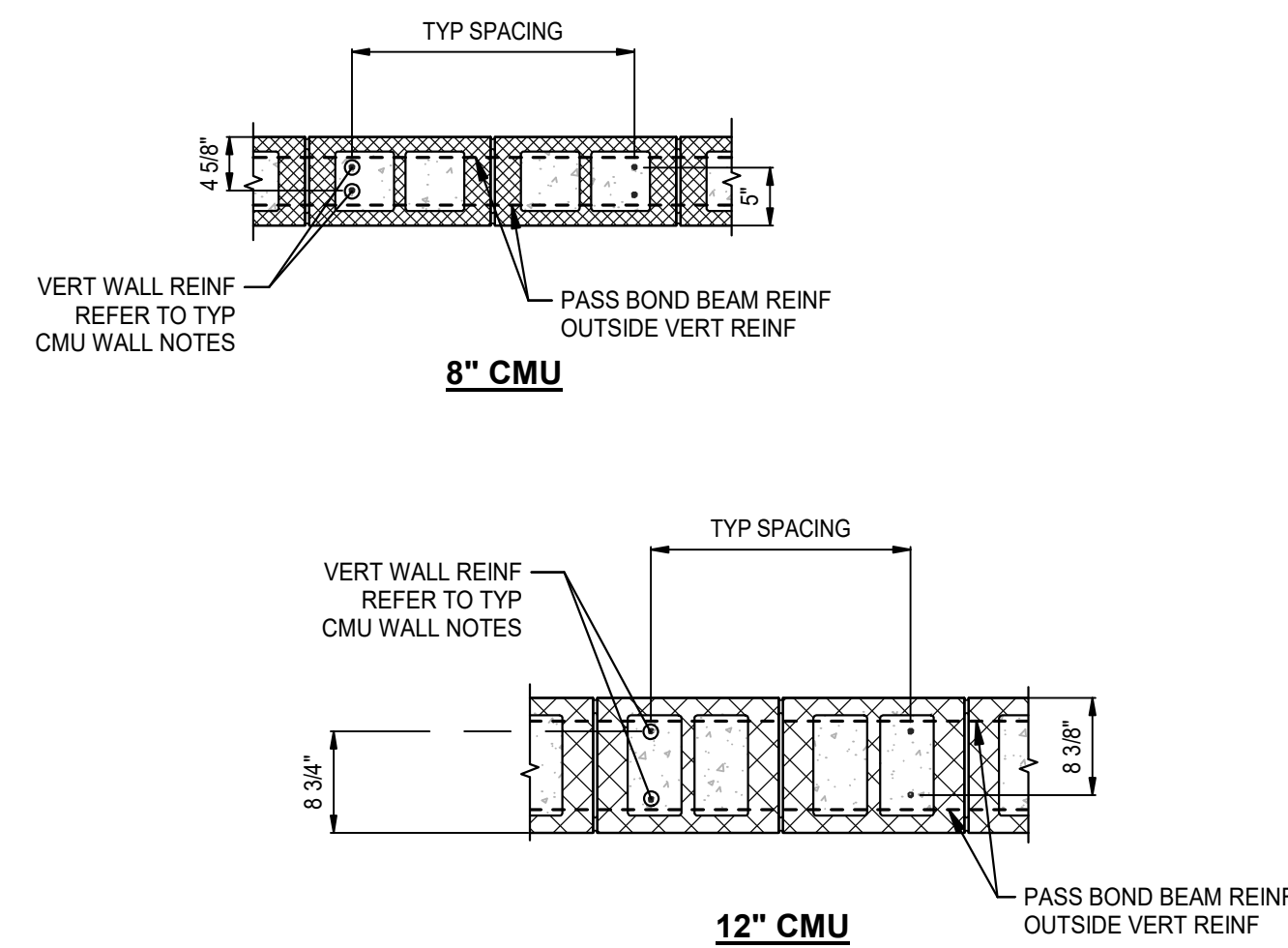
**2** CHANGES IN BOND BEAM ELEVATION  
S3.3 3/4" = 1'-0"



**3** NON-TOOTHED CMU WALL INTERSECTIONS  
S3.3 3/4" = 1'-0"



**4** TYPICAL CMU WALL CORNERS  
S3.3 3/4" = 1'-0"



NOTE:  
DOUBLE REINF WALLS ARE INDICATED BY CALLING FOR (2) BARS @ TYP SPACING.  
**5** DOUBLE REINF CMU WALL DETAIL  
S3.3 3/4" = 1'-0"

**M M**  
**MOTT**  
**MACDONALD**

107 St. Francis Street  
Suite 2900,  
Mobile, Alabama 36602  
Telephone: (251) 343-4368  
Fax: (251) 343-6902  
Architects  
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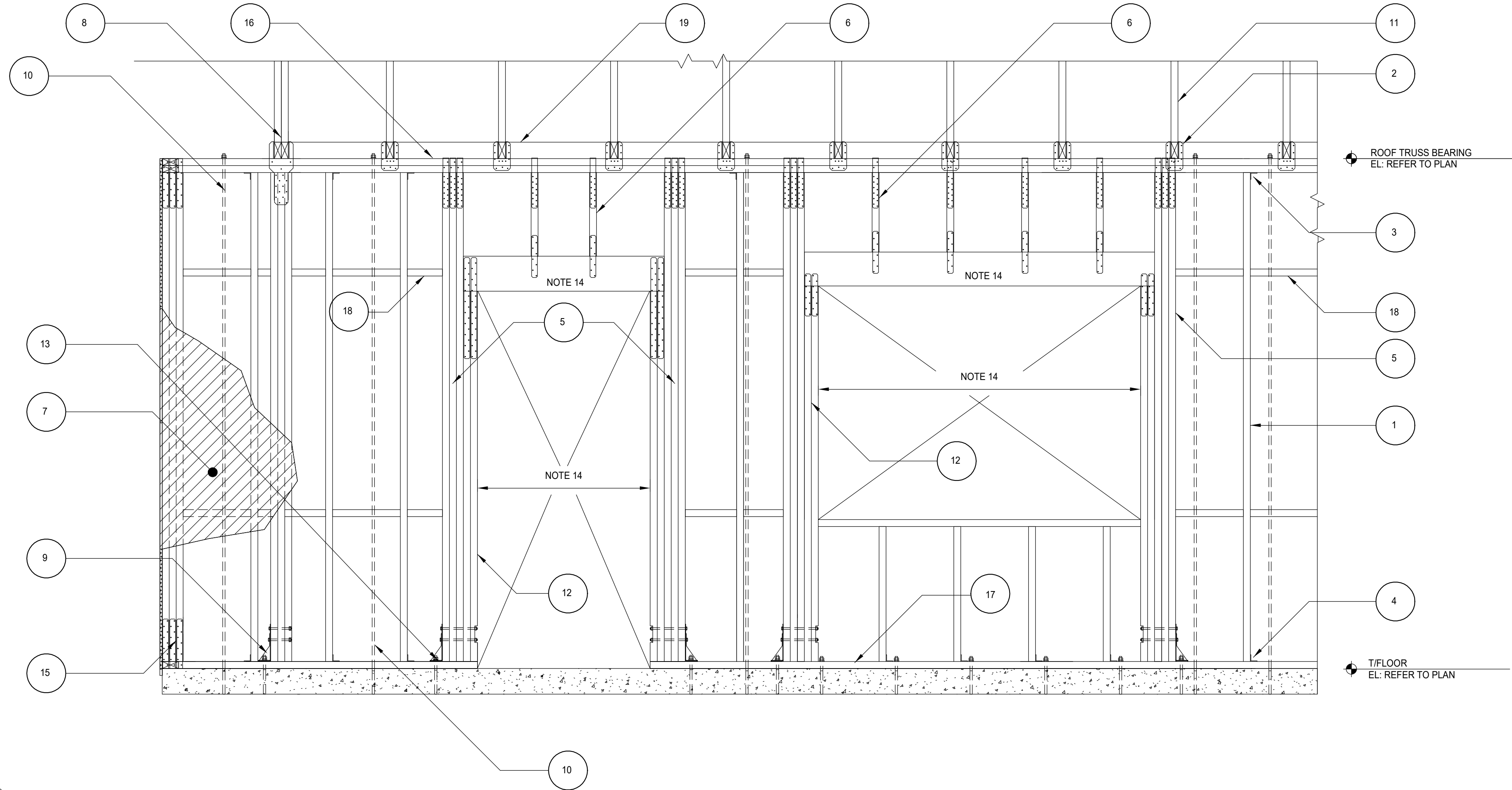
SCALE 3/4" = 1'-0"

ISSUED FOR PERMIT



DATE: May 5, 2024

S3.3



**1** LOAD BEARING WALL FRAMING  
 S3.4 3/4" = 1'-0"

**TYPICAL EXTERIOR WALL NOTES**

1. 2x6 NO. 1 SOUTHERN PINE STUDS AT 16" OC.
2. JOISTS TO WALL CONNECTION SHALL BE AS INDICATED ON S1.5 AND S1.5.
3. 2x6 WALL STUD TO TOP PLATE W/ (4) 16d GALV. RING SHANK NAILS.
4. 2x6 WALL STUD TO BOTTOM PLATE W/ (4) 16d GALV. RING SHANK NAILS.
5. (4)2x6 JAMB STUDS EACH SIDE OF OPENINGS, UON. CONNECT STUD W/ SPH8.
6. JACK STUDS AT 16" OC W/ (4) 16d GALV RING SHANK NAILS TO TOP PLATE AND SIMPSON MSTA24 STRAP TO HEADER.
7. 3/4" APA RATED STRUCTURAL 1 EXTERIOR GRADE SHEATHING EACH SIDE (BLOCK ALL EDGES) W/10d NAILS AT 4" OC EDGES AND 12" OC INTERMEDIATE SUPPORTS, ONE SIDE TYPICAL.
8. LVL GIRDER ROOF JOISTS WITH A H10A-2 HOLDDOWN AT EACH LVL GIRDER ROOF JOISTS.
9. (4)2x6 STUDS UNDER ALL LVL ROOF JOISTS MEMBERS WITH SIMPSON HD7B HOLDDOWN (7/8" Ø GALV THREADED ROD THRU WALL PLATE BELOW W/ NUT AND WASHER).
10. 3/4" Ø GALV. THREADED ROD FULL HEIGHT OF WALL W/ GALV. 1/8"x2"x2" PLATE WASHERS AND NUTS EA. END. RODS SHALL EXTEND FROM THE FOUNDATION TO THE TOP PLATE @ 4'-0" O.C MAX.
11. ROOF JOISTS @ 16" OC MAX.
12. (2)2x6 HEADER STUDS EACH SIDE OF OPENINGS W/(2) SIMPSON ST22 STRAPS AT HEADER AND (2) SPH6 TIES OR HOLDDOWNS AT BOTTOM PLATE.
13. SIMPSON HD3B HOLDDOWN (5/8" Ø GALV. THREADED ROD THRU WALL PLATE BELOW W/ NUT AND WASHER).
14. REFER TO S1.4 AND S1.6 FOR HEADER MEMBER SIZES.
15. (3)2x6 STUDS AT ENDS OF ALL EXTERIOR WALLS CONNECT TO BOTTOM PLATE W/ ST24 AT EACH STUD.
16. TOP PLATE ASSEMBLY: DOUBLE 2x6 AND BEVELED 4x6 TOP PLATE TO MATCH ROOF SLOPE. TOP PLATE ASSEMBLY SHALL BE CONNECTED WITH GALV. 1/2" DIA THREADED ROD WITH GALV. NUTS AND WASHERS @ 24" O.C.
17. 2x6 PRESSURE TREATED BOTTOM PLATE.
18. 2x FULL-DEPTH BLOCKING AT ALL SHEATHING EDGES.
19. 2x FULL-DEPTH BLOCKING @ ROOF JOISTS
20. ALL STRAPS AND METAL COMPONENTS SHALL BE GALVANIZED TO A G90 COATING THICKNESS UNLESS NOTED OTHERWISE.

**M**  
**MOTT**  
**MACDONALD**

107 St. Francis Street  
 Suite 2900,  
 Mobile, Alabama 36602  
 Telephone: (251) 343-4326  
 Fax: (251) 343-6902  
 Architects  
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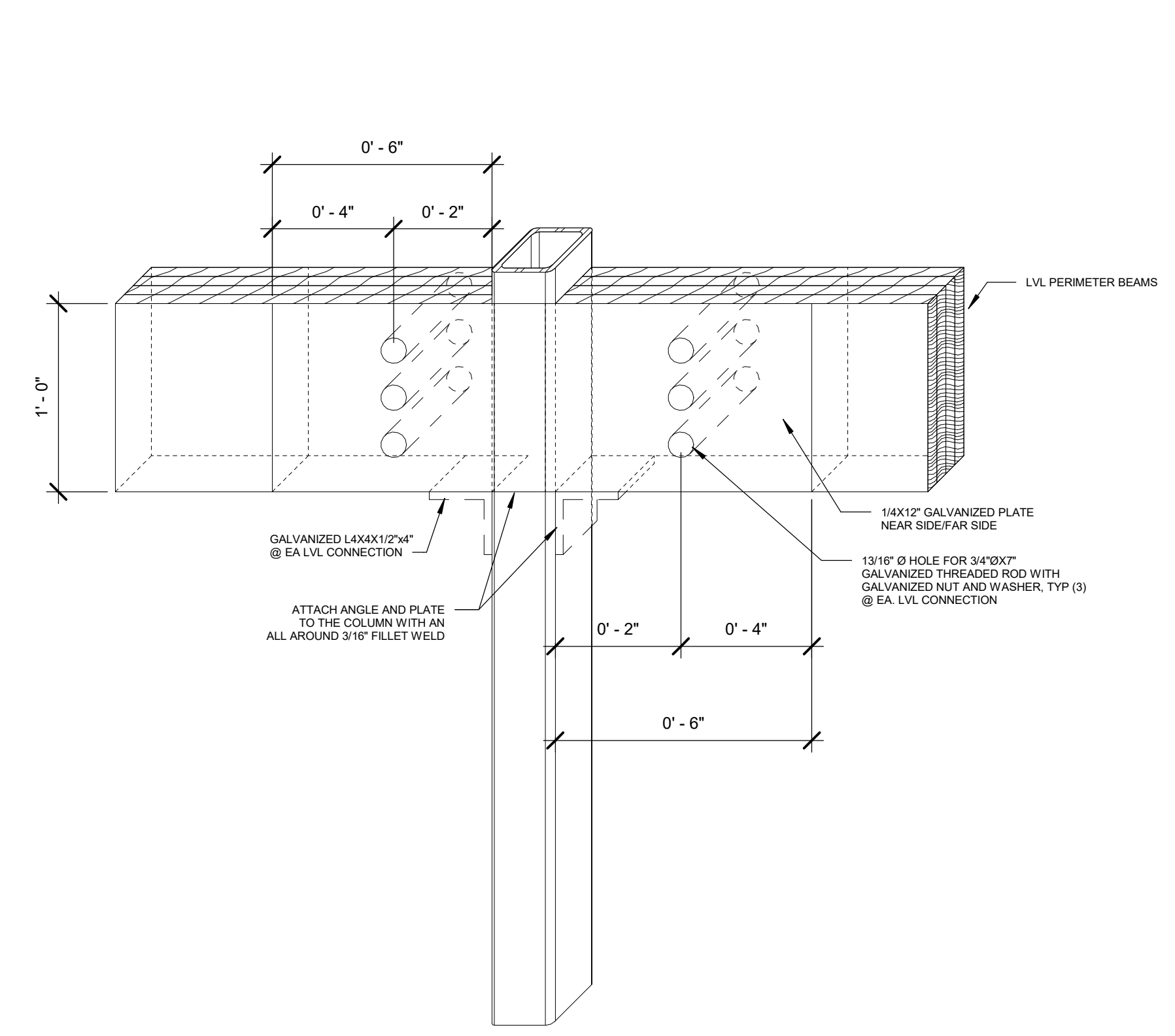
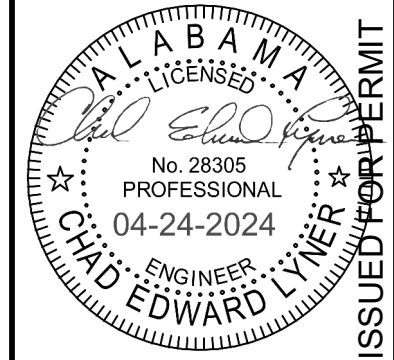
ARCHITECTURAL DRAWINGS FOR:  
**CITY OF MOBILE - MIMS PARK CONCESSIONS**  
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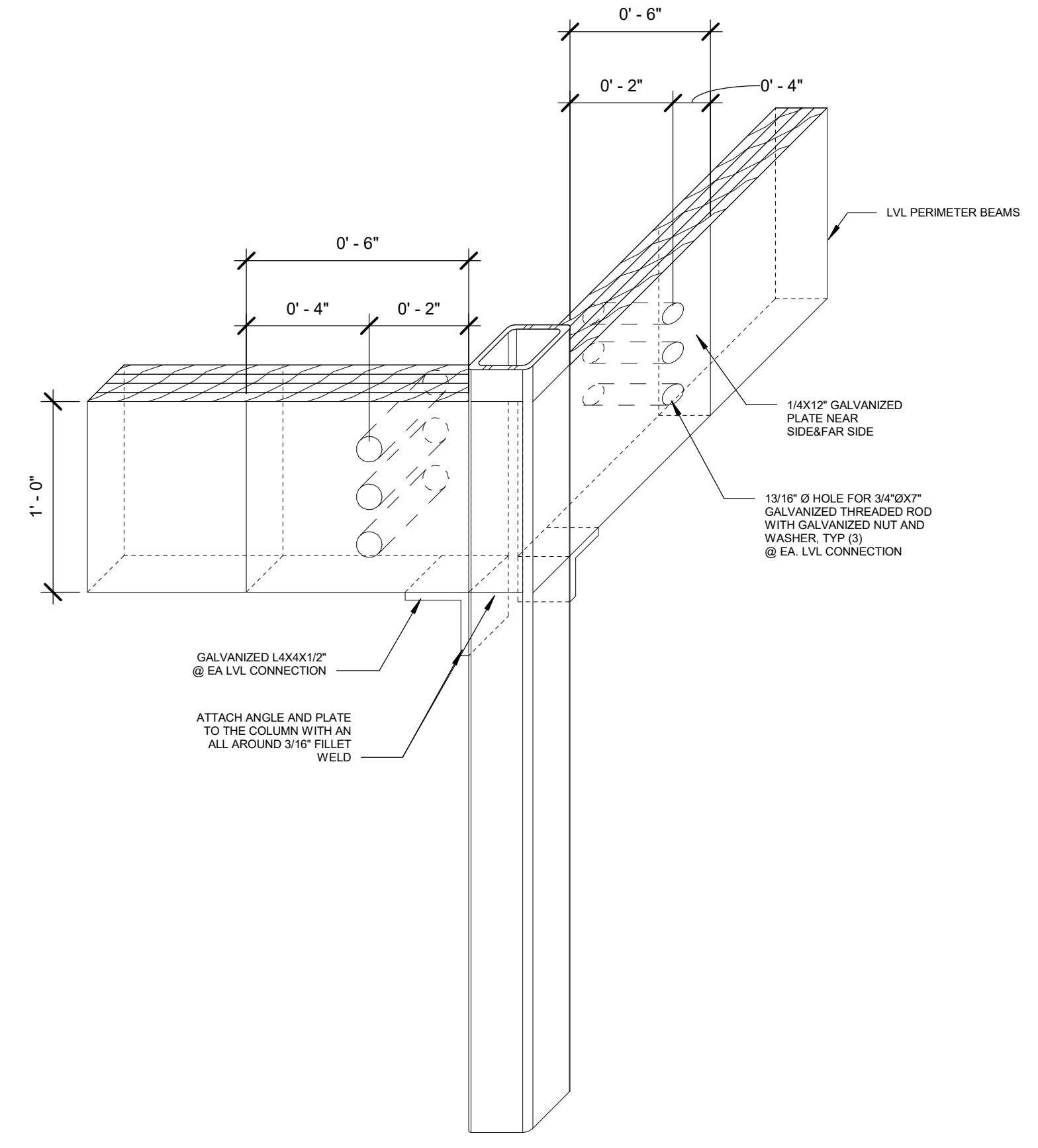
DATE: May 5, 2024  
 SCALE: 3/4" = 1'-0"  
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S3.4

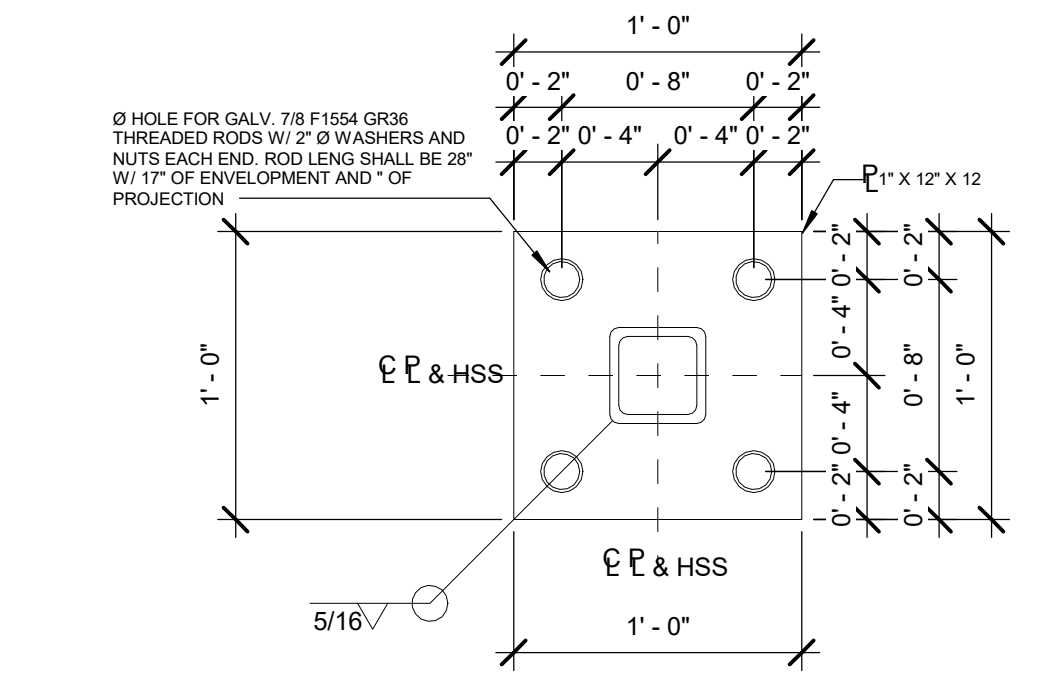




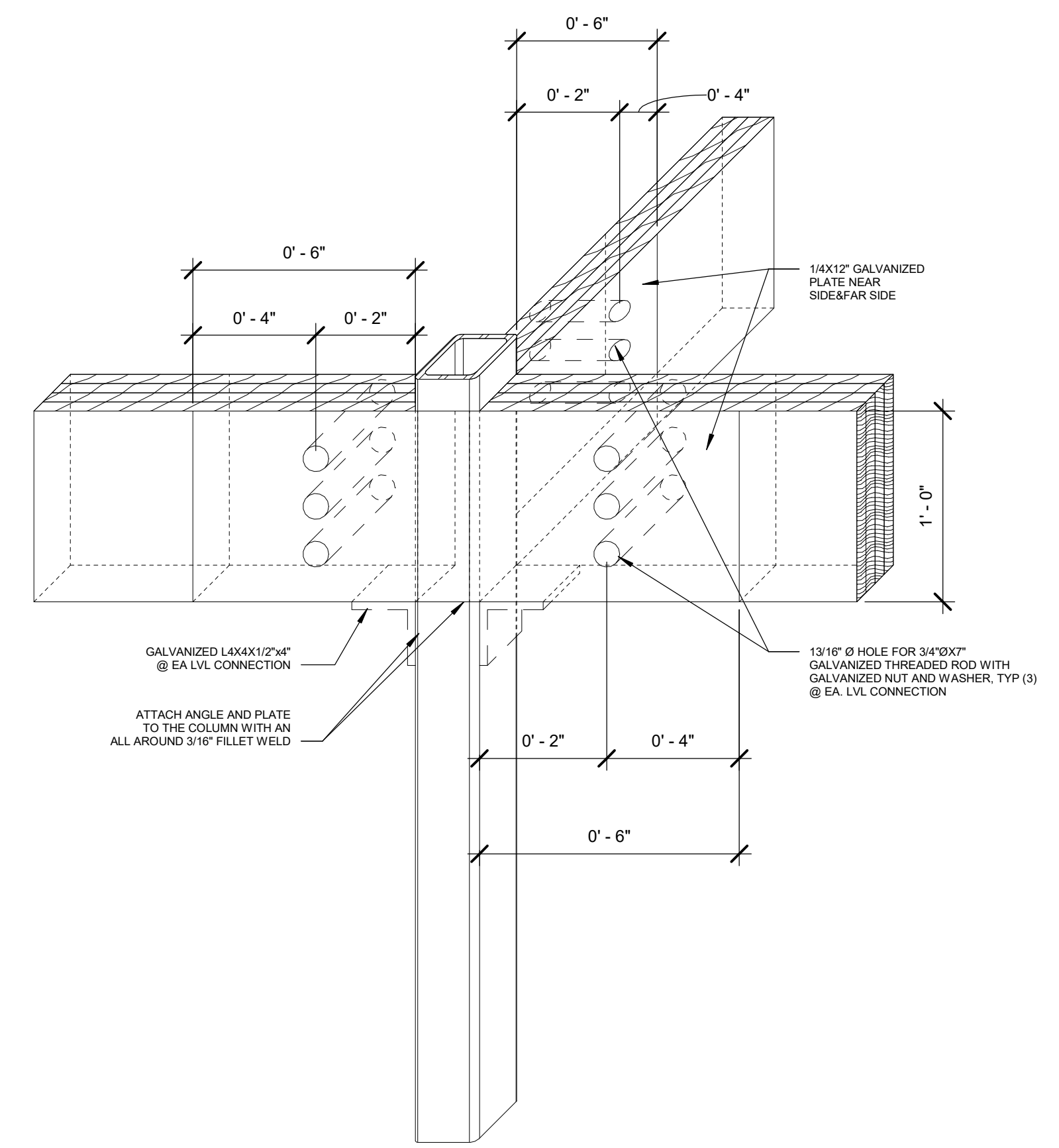
**1** 2 BEAM CONNECTION  
 S3.5 1 1/2" = 1'-0"



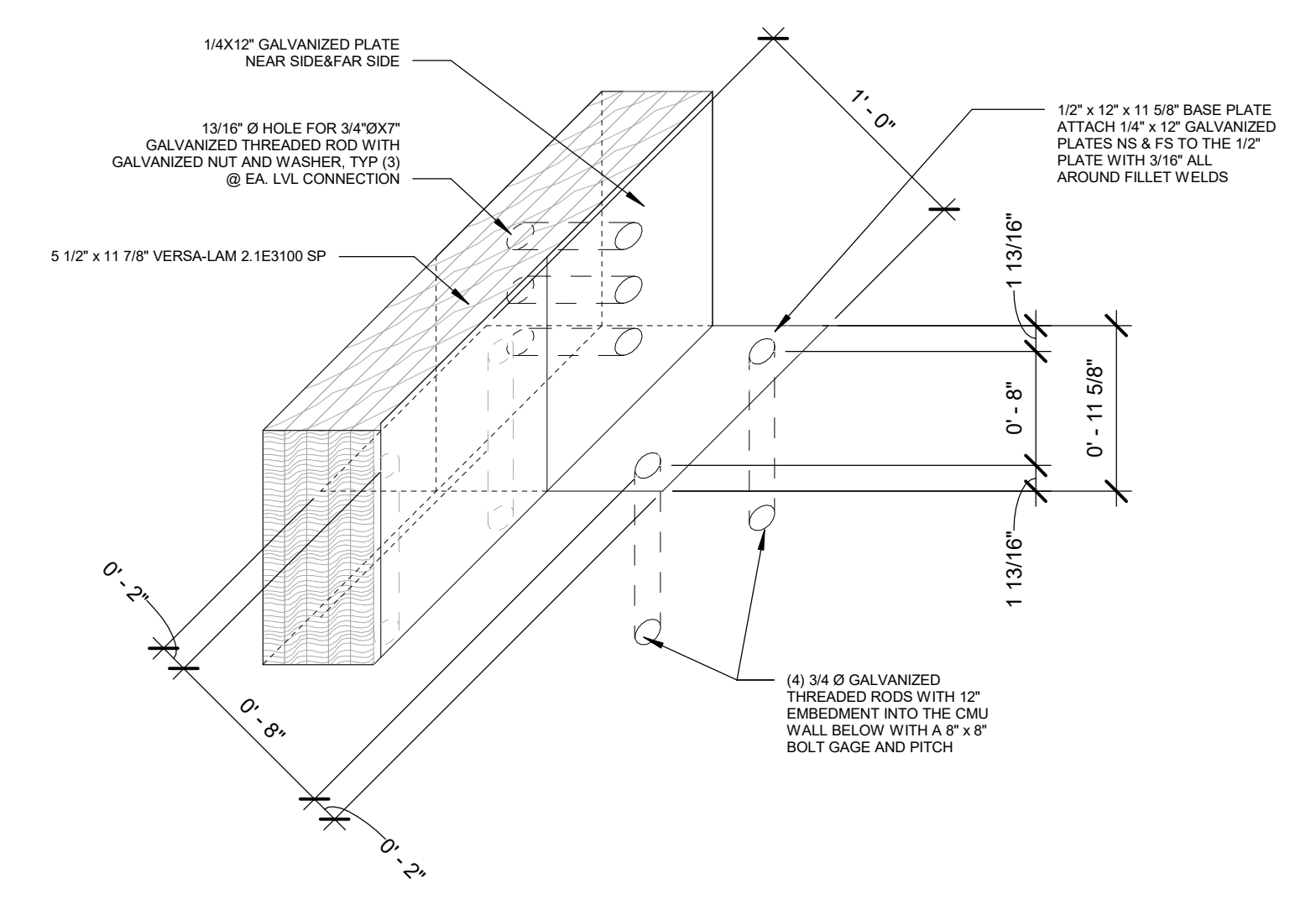
**2** 2 BEAM 90 DEGREE CONNECTION  
 S3.5 1 1/2" = 1'-0"



STEEL BASEPLATE PLAN DETAIL  
 SCALE: 1 1/2" = 1'-0"



**3** 3 BEAM T CONNECTION  
 S3.5 1 1/2" = 1'-0"



**4** BEAM TO CMU WALL CONNECTION  
 S3.5 1 1/2" = 1'-0"